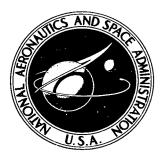
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MEMBRANE ANALYSIS OF SCALLOPED SHELLS

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NATIONAL AERONAUTICS AND SPACE ADMINISTRATION

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SUMMARY

The main objective of this report is to contribute to the understanding of membrane forces in scalloped shells, particularly scalloped paraboloids. It is shown how the stress function approach, used successfully in the membrane analysis of elliptic and hyperbolic paraboloids, can be applied to the scalloped paraboloids. In order to demonstrate the technique, two scalloped structures having practical significance as structural models for parachutes are discussed in detail. In the cases considered, it is shown that the stress function must satisfy a linear second-order partial-differential equation with variable coefficients. The basic method for solving this equation is given in the first appendix. For comparison purposes, numerical results for similar shells of revolution are also presented. These results show that the hoop and meridional stresses in the parachute type structure are lower than those in a corresponding shell of revolution. More significantly, the meridional stress in the parachute type shell decreases remarkably, and even becomes compressive, as the bulge of the scallop, defined by a positive parameter β , is increased. It is shown how a critical value, β_{CP} , of this bulge parameter can be obtained below which the shell would remain in a compressionless state. A singular solution used in prescribing rope tension near the opening of the parachute type shell may also be useful in other aspects of parachute analysis.

A brief review of the scope and the limitations of the method and the possibility of extending it to other scalloped shells of the same family, is presented in a separate section. Suggestions for further work are included in the concluding remarks.

INTRODUCTION

The advent of the space age is largely responsible for a growing interest in new structural concepts and techniques. Promising among the newer concepts are those of filament-wound structures and expandable structures. With these advances in technology, there exists a parallel need for theoretical investigations of different structural forms. The membrane analysis of one such form, the scalloped shell, is presented in this report.

As opposed to shells of revolution, scalloped shells have been relatively unexplored as structural forms. The parachute is, perhaps, the only form of scalloped structure that has been studied in any detail. An interesting history of the parachute and its development as a decelerating device is given in reference 1, which contains, in addition, valuable information

about various aspects of parachute performance and design. The earliest published attempts at parachute stress analysis date from 1923 when the work of Taylor, Southwell, Griffiths, Jones, and Williams, on ideal shapes of parachutes, was compiled by R. Jones in reference 2. Since then, several authors have continued this work, studying the many different aspects of parachute performance and design.

Most of the work concerning parachutes has been based on certain simplifying assumptions regarding the character of stresses in the surface. These assumptions can be broadly classified into three categories as follows:
(a) The circumferential stress in the parachute is either negligible (ref. 2) or negligible within a certain critical radius (ref. 3). This position was taken by the early investigators; (b) It is the meridional stress which is negligible rather than the hoop stress (refs. 4 and 5); (c) The circumferential stress is proportional to the product of the applied pressure and a reference radius. The meridional stress is assumed either negligible, for certain cases, or obtainable from the conditions of equilibrium (ref. 6).

The present work differs from parachute stress analysis in two important respects. First, the emphasis here is on the membrane analysis of scalloped surfaces as shell structures. The shells discussed in detail belong to the general class of surfaces designated as bulged or scalloped paraboloids, of which the parachute type structure is only one member. Secondly, since the investigation is based on the membrane theory of shells, no a priori assumptions are necessary regarding the nature of stresses – and none are made.

The method of analysis relies on the stress function approach postulated in the membrane theory of shells of arbitrary shape. This stress function approach has been successfully used in the past for solving the problems of elliptic and hyperbolic paraboloids, and it is shown in this report that this method can also be successfully applied to a more general class of scalloped shells, which includes the scalloped paraboloids as special cases. In order to demonstrate the method, the solutions for two different types of scalloped structures are discussed in detail. Since the loading and boundary conditions prescribed for these two structures could also apply to comparable parachutes, it is conceivable that these structures could serve as structural models for parachutes.

The technique used in this paper can also be applied to a wider variety of loading and edge conditions than those considered in the two examples. A more detailed discussion of the scope and the limitations of the method is given in a separate section.

NOTATION

- A surface parameter used to define the scalloped paraboloids
- A_{m} undetermined coefficients associated with the homogeneous solutions Ψ_{Hm}

Ι

a reference radius, also maximum radius

coefficients of a power series in x (appendix B) $a_{m,n}$ \mathbb{B}_{m} undetermined coefficients associated with the homogeneous solutions Ψ_{Hm} ъ radial distance to the inner opening coefficients of power series in x (appendix A) $C_{m,n},D_{m,n}$ $F(\rho_{\bullet}x)$ function of p and x; denotes right hand side of an inhomogeneous partial differential equation $F_{m}(x), F_{\alpha}(x), F_{\beta}(x), F_{\gamma}(x)$ functions of x $f_{m}(x)$ function of x normal component of the forces acting on the rope (fig. 9) f_n $f_{\alpha}(x), f_{\beta}(x)$ functions of x $g_{m}(x),g_{\alpha}(x),$ $g_{\beta}(x)$ functions of x normal distance from the apex of the shell to a horizontal h plane passing through the point $(r = a, \theta = 0)$ constant associated with a singular solution; also a subscript k critical value of the constant k k_{cr} linear differential operator \mathbf{L} summation index; also subscript \mathbf{m} number of segments or gores N membrane forces per unit length in the tangential plane of $N_{r}, N_{\theta}, N_{r\theta}$ the shell (fig. 2) $\overline{N}_r, \overline{N}_\theta, \overline{N}_{r\theta}$ projected membrane forces (fig. 2) summation index; also denotes a power of r and powers of x n dimensionless membrane forces nr,na,nre concentrated load at the apex in the Z direction P reference load intensity, also uniform normal pressure р r, θ, z components of the load intensity p_r, p_θ, p_z

```
\overline{p}_r, \overline{p}_\theta, \overline{p}_z
              reduced load intensities
Q(x)
              function of x (appendix B)
              dimensionless load intensities
 q_r, q_\theta, q_z
R(x)
              function of x (appendix B)
              polar distance in cylindrical coordinates (fig. 1), also a
                 subscript
              summation index
 s
T
              rope tension
              function of x (appendix A)
u_{m}(x)
x
              transformed coordinate corresponding to the angular distance \theta
                in polar coordinates
              constant used in defining the range of the variable x
\mathbf{x}_{O}
y(x)
              function of x (appendix B)
Z(r,\theta)
              vertical coordinate defined as a function of polar coordinates
                r,\theta to define the middle surface of the shell
              angle between a tangent to the middle surface and the r direc-
                tion; also used as a subscript and to denote a power of the
                variable ρ
\alpha_n, \alpha_n, \alpha_n
              coefficients of powers of x in the series solutions
β
             bulge parameter used in defining the scalloped paraboloids; also
                used as a subscript
Ber
              critical value of the bulge parameter
\beta_n, \beta_n', \beta_n''
              coefficients of powers of x in the series solutions
             angle between a tangent to the middle surface and the \theta direc-
                tion; also used as a subscript, denotes power of \rho
             coefficients of powers of x (appendix A)
\gamma_n, \gamma_n'
δ
             denotes a power of p
             coefficients of powers of x (appendix B)
\delta_{\mathbf{n}}
θ
             angular distance in cylindrical coordinates (fig. 1)
```

 θ_{0} constant used in defining the range of the variable θ $\lambda_{\mathbf{k}}$ used to denote powers of p in the separated solution $\lambda_{\mathbf{m}}$ eigenvalues constant, describes the ratio of height h to the maximum radius a μ coefficients of power series in x (appendix B) μ_n constants of integration (appendix A) $\mu_{\alpha}, \mu_{\beta}$ constant related to prescribed edge forces (fig. 10(b)) Ę dimensionless radial distance ρ dimensionless radius corresponding to an opening at r = b ρ_{O} $\Phi(\mathbf{r},\theta)$ postulated stress function homogeneous solution corresponding to the separated form $\rho^{\Lambda_k} \phi_{lr}(x)$ $\varphi_{k}(\mathbf{x})$ $\Psi(\rho,x)$ dimensionless stress function solution of a homogeneous differential equation subject to Ψ_{H} homogeneous boundary conditions homogeneous solutions corresponding to eigenvalues λ_m Ψ_{Hm} $q\Psi$ particular solution of an inhomogeneous differential equation subject to inhomogeneous boundary conditions

Subscripts

cr critical values of parameters

H homogeneous solution

Hm individual components of homogeneous solution

p particular solution

MEMBRANE EQUATIONS

Consider a thin shell whose middle surface is defined by the relationship $Z = Z(r,\theta)$

which expresses the vertical coordinate Z of the middle surface as a function of the polar coordinates, r,θ (fig. 1). A differential element of this surface is shown enlarged in figure 2. The element is contained between two adjacent vertical planes, θ = constant, and two adjacent vertical cylinders, r = constant. It is acted upon by the loads p_r , p_θ , p_z (per unit area of the middle surface) and supported by the membrane forces N_r , N_θ , $N_r\theta$. The latter quantities are forces per unit length of the line element through which they are transmitted.

The membrane equations, which N_r , N_θ , $N_{r\theta}$ must satisfy in order to assure equilibrium, can be conveniently written in terms of the projected stress resultants \overline{N}_r , \overline{N}_θ , $\overline{N}_{r\theta}$ and the reduced load intensities \overline{p}_r , \overline{p}_θ , \overline{p}_z as follows:

$$\frac{1}{r} \frac{\partial}{\partial r} (r \overline{N}_r) + \frac{1}{r} \frac{\partial}{\partial \theta} (\overline{N}_{r\theta}) - \frac{\overline{N}_{\theta}}{r} = -\overline{p}_r \quad (1a)$$

$$\frac{1}{r} \frac{\partial}{\partial \theta} (\overline{N}_{\theta}) + \frac{\partial}{\partial r} (\overline{N}_{r\theta}) + 2 \frac{\overline{N}_{r\theta}}{r} = -\overline{p}_{\theta}$$
 (1b)

$$\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial Z}{\partial r} \overline{N}_{r} \right) + \frac{1}{r} \frac{\partial}{\partial r} \left(\frac{\partial Z}{\partial \theta} \overline{N}_{r\theta} \right) + \frac{1}{r} \frac{\partial}{\partial \theta} \left(\frac{\partial Z}{\partial r} \overline{N}_{r\theta} \right) + \frac{1}{r} \frac{\partial}{\partial \theta} \left(\frac{1}{r} \frac{\partial Z}{\partial \theta} \overline{N}_{\theta} \right) = -\overline{p}_{Z}$$
 (lc)

where

$$\overline{N}_{r} = \frac{\cos \alpha}{\cos \gamma} N_{r}$$

$$\overline{N}_{\theta} = \frac{\cos \gamma}{\cos \alpha} N_{\theta}$$

$$\overline{N}_{r\theta} = N_{r\theta}$$
(2)

$$\frac{\overline{p}_{r}}{\overline{p}_{r}} = \frac{\left(\cos^{2} \gamma + \cos^{2} \alpha \sin^{2} \gamma\right)^{1/2}}{\cos \alpha \cos \gamma} \qquad p_{r}$$

$$\frac{\overline{p}_{\theta}}{\overline{p}_{\theta}} = \frac{\left(\cos^{2} \gamma + \cos^{2} \alpha \sin^{2} \gamma\right)^{1/2}}{\cos \alpha \cos \gamma} \qquad p_{\theta}$$

$$\frac{\overline{p}_{z}}{\overline{p}_{z}} = \frac{\left(\cos^{2} \alpha + \cos^{2} \alpha \sin^{2} \gamma\right)^{1/2}}{\cos \alpha \cos \gamma} \qquad p_{z}$$

$$\tan \alpha = \frac{\partial Z}{\partial r}$$

$$\tan \gamma = \frac{1}{r} \frac{\partial Z}{\partial \rho}$$
(3)

Projected on a plane z = const. (fig. 2).

With the introduction of the following dimensionless stress resultants and load intensities,

$$n_{\mathbf{r}} = \frac{\overline{N}_{\mathbf{r}}}{pa}$$

$$n_{\theta} = \frac{\overline{N}_{\theta}}{pa}$$

$$n_{\mathbf{r}\theta} = \frac{\overline{N}_{\mathbf{r}\theta}}{pa}$$

$$(4)$$

$$q_{\mathbf{r}} = \frac{\overline{p}_{\mathbf{r}}}{p}$$

$$q_{\theta} = \frac{\overline{p}_{\theta}}{p}$$

$$q_{z} = \frac{\overline{p}_{z}}{p}$$
(5)

where p is a reference load intensity and a is a reference radius $\geq r$, the equations (la) - (lc) can be written as follows:

$$\frac{\partial}{\partial \mathbf{r}} (\mathbf{n_r}) + \frac{1}{r} \frac{\partial}{\partial \theta} (\mathbf{n_{r\theta}}) + \frac{\mathbf{n_r} - \mathbf{n_{\theta}}}{r} = -\frac{\mathbf{q_r}}{a}$$
 (6a)

$$\frac{1}{r} \frac{\partial}{\partial \theta} (n_{\theta}) + \frac{\partial}{\partial r} (n_{r\theta}) + 2 \frac{n_{r\theta}}{r} = -\frac{q_{\theta}}{a}$$
 (6b)

$$\left(\frac{\partial^{2}Z}{\partial \mathbf{r}} + \frac{1}{\mathbf{r}} \frac{\partial Z}{\partial \mathbf{r}}\right) \mathbf{n}_{\mathbf{r}} + \frac{2}{\mathbf{r}} \frac{\partial^{2}Z}{\partial \mathbf{r} \partial \theta} \mathbf{n}_{\mathbf{r}\theta} + \frac{1}{\mathbf{r}^{2}} \frac{\partial^{2}Z}{\partial \theta^{2}} \mathbf{n}_{\theta} + \frac{\partial Z}{\partial \mathbf{r}} \frac{\partial \mathbf{n}_{\mathbf{r}}}{\partial \mathbf{r}} + \frac{1}{\mathbf{r}} \frac{\partial Z}{\partial \theta} \frac{\partial \mathbf{n}_{\mathbf{r}\theta}}{\partial \mathbf{r}} + \frac{1}{\mathbf{r}^{2}} \frac{\partial Z}{\partial \theta} \frac{\partial \mathbf{n}_{\theta}}{\partial \theta} = -\frac{\mathbf{q}_{Z}}{\mathbf{a}} \tag{6c}$$

Equations (6a) - (6c) are applicable to any shell surface of arbitrary shape, in cylindrical coordinates, so long as there exists a one-to-one correspondence between points of the middle surface of the shell and their projections on a horizontal plane (Z = const.). If the forces n_r , n_θ , $n_{r\theta}$ are defined in terms of an Airy type stress function $\Phi(r,\theta)$ such that

.

$$n_{\mathbf{r}} = \frac{1}{r} \frac{\partial \Phi}{\partial \mathbf{r}} + \frac{1}{r^{2}} \frac{\partial^{2} \Phi}{\partial \theta^{2}} - \frac{1}{r} \int \mathbf{r} \frac{\mathbf{q}_{\mathbf{r}}}{\mathbf{a}} d\mathbf{r} - \frac{1}{r} \int \mathbf{r} \int \frac{\mathbf{q}_{\theta}}{\mathbf{a}} d\theta d\mathbf{r}$$

$$n_{\theta} = \frac{\partial^{2} \Phi}{\partial \mathbf{r}^{2}} - \mathbf{r} \int \frac{\mathbf{q}_{\theta}}{\mathbf{a}} d\theta$$

$$n_{r\theta} = \frac{1}{r^{2}} \frac{\partial \Phi}{\partial \theta} - \frac{1}{r} \frac{\partial^{2} \Phi}{\partial \mathbf{r} \partial \theta}$$

$$(7)$$

then, it can be verified that equations (6a) and (6b) are automatically satisfied and equation (6c) reduces to:

$$\frac{\left(\frac{1}{r}\frac{\partial Z}{\partial r} + \frac{1}{r^2}\frac{\partial^2 Z}{\partial \theta^2}\right)}{\frac{\partial^2 \Phi}{\partial r^2}} + \left(\frac{2}{r^3}\frac{\partial Z}{\partial \theta} - \frac{2}{r^2}\frac{\partial^2 Z}{\partial r\partial \theta}\right) \frac{\partial^2 \Phi}{\partial r\partial \theta}} + \left(\frac{1}{r^2}\frac{\partial^2 Z}{\partial r^2}\right) \frac{\partial^2 \Phi}{\partial \theta^2} + \left(\frac{1}{r}\frac{\partial^2 Z}{\partial r^2}\right) \frac{\partial \Phi}{\partial r} + \left(\frac{2}{r^3}\frac{\partial^2 Z}{\partial r\partial \theta} - \frac{2}{r^4}\frac{\partial Z}{\partial \theta}\right) \frac{\partial \Phi}{\partial \theta}} = -\frac{q_Z}{a} + \frac{\partial Z}{\partial r}\frac{q_r}{a} + \frac{1}{r}\frac{\partial Z}{\partial \theta}\frac{q_\theta}{a} + \left(\frac{1}{r}\frac{\partial^2 Z}{\partial \theta^2} + \frac{\partial Z}{\partial r}\right)\int \frac{q_\theta}{a} d\theta} + \frac{\partial^2 Z}{\partial r^2}\left(\frac{1}{r}\int r\frac{q_r}{a} dr + \frac{1}{r}\int r dr\int \frac{q_\theta}{a} d\theta\right) (8)^2$$

The governing equation (8) is a second-order partial differential equation, and, depending on the choice of surface, may have either constant or variable coefficients. The stress function approach has been successfully used in the past for solving problems of elliptic and hyperbolic paraboloids. For each of these cases equation (8) has constant coefficients and is reducible to a standard form: the former to the Poisson's equation, the latter to the wave equation. In the case of the scalloped paraboloids, equation (8) has variable coefficients, as will be seen later.

THE SCALLOPED PARABOLOID

The middle surface of the scalloped or bulged paraboloids may be defined by the relationship

$$Z = Ar^{2}(1 + \beta\theta^{2}) , \quad -\theta_{0} \le \theta \le \theta_{0} , \quad \theta_{0} \le \pi , \quad \beta > 0$$
 (9)³

²Reference 7 gives a similar formulation in rectangular coordinates. ³The shell surface, it may be noted, is symmetric with respect to the plane $\theta = 0$.

where A, β are constants. The parameter β defines the bulge of the scalloped surface and may be called the bulge parameter. It should be noted that when β equals zero, equation (9) describes a paraboloid of revolution, hence, the designation "bulged" or "scalloped" paraboloid.

Such a shell, like any other structure, can be loaded in many different ways. As a roof structure, it may be loaded by gravitational and wind loads. As a pressure vessel bulkhead, it may be subjected to a uniform or varying normal pressure. As a parachute type structure, a uniform normal pressure would constitute a reasonable loading. In the analysis to follow, a uniform normal pressure will be assumed.

Assuming the surface segment defined by equation (9), then the load intensities, the definitions (3) and (4), and the equilibrium condition (8) take the form

$$p_{r} = \frac{\sin \alpha \cos \gamma}{(\cos^{2} \alpha + \cos^{2} \alpha \sin^{2} \gamma)^{1/2}} p$$

$$p_{\theta} = \frac{\cos \alpha \sin \gamma}{(\cos^{2} \alpha + \cos^{2} \alpha \sin^{2} \gamma)^{1/2}} p$$

$$p_{z} = -\frac{\cos \alpha \cos \gamma}{(\cos^{2} \alpha + \cos^{2} \alpha \sin^{2} \gamma)^{1/2}} p$$

$$\overline{p}_{r} = \tan \alpha p$$

$$\overline{p}_{\theta} = \tan \gamma p$$

$$\overline{p}_{z} = -p$$

$$q_{r} = \tan \alpha$$

$$q_{\theta} = \tan \gamma$$

$$q_{z} = -1$$

$$\tan \alpha = 2Ar(1 + \beta\theta^{2})$$

where

$$\tan \gamma = 2Ar\beta\theta$$

and

$$(1 + \beta + \beta\theta^2) \frac{\partial^2 \Phi}{\partial r^2} + (1 + \beta\theta^2) \frac{1}{r^2} \frac{\partial^2 \Phi}{\partial \theta^2} - 2\beta\theta \cdot \frac{1}{r} \frac{\partial^2 \Phi}{\partial r \partial \theta}$$

+
$$(1 + \beta\theta^2) \frac{1}{r} \frac{\partial \Phi}{\partial r} + 2\beta\theta \cdot \frac{1}{r^2} \frac{\partial \Phi}{\partial \theta}$$

$$= \frac{1}{2Aa} + \frac{Ar^2}{a} \left[\frac{8}{3} + \left(\frac{20}{3} + 3\beta \right) \beta \theta^2 + 4\beta^2 \theta^4 \right]$$
 (11)

Next, introduce the transformation

$$\rho = r/a
x = \sqrt{\beta} \theta$$
(12)

where a is a reference radius, and define

$$\Phi(\mathbf{r},\theta) = \mathbf{a}^2 \Psi(\mathbf{p},\mathbf{x}) \tag{13}$$

$$\begin{array}{ll}
h = \mu a \\
\mu = constant
\end{array}$$
(14)

The coordinate ρ in equations (12) is a dimensionless radius; the coordinate x, which is linearly related to θ , is introduced for convenience in algebraic and numerical work, and the stress function Ψ in equation (13) is a dimensionless function. The height h, designated as the normal distance from the apex to a horizontal plane passing through the point r = a, $\theta = 0$, is defined in equations (14) as a certain multiple of the reference radius a. If equations (12) - (14) are substituted into equation (11), it can be shown that the stress function $\Psi(\rho, \mathbf{x})$ must satisfy the differential equation:

$$L(\Psi) = F(\rho, \mathbf{x}) \tag{15}$$

where L is the linear differential operator defined by

$$L = \left[(1 + \beta + x^2) \frac{\partial^2}{\partial \rho^2} + (1 + x^2) \frac{1}{\rho} \frac{\partial}{\partial \rho} + 2\beta x \frac{1}{\rho^2} \frac{\partial}{\partial x} \right]$$

$$(16)$$

and

$$F(\rho, x) = \frac{1}{2\mu} + \mu \rho^2 \left[\frac{8}{3} + \left(3\beta + \frac{20}{3} \right) x^2 + \mu x^4 \right]$$
 (17)

In terms of the dimensionless stress function Ψ and the variables ρ, x the membrane forces n_r , n_θ , $n_{r\theta}$ can now be expressed as follows:

$$n_{\mathbf{r}\theta} = \frac{1}{\rho} \frac{\partial \Psi}{\partial \rho} + \frac{\beta}{\rho^{2}} \frac{\partial^{2} \Psi}{\partial x^{2}} - \mu \rho^{2} \left(\frac{2}{3} + x^{2} \right)$$

$$n_{\theta} = \frac{\partial^{2} \Psi}{\partial \rho^{2}} - \mu \rho^{2} x^{2}$$

$$n_{\theta} = \sqrt{\beta} \left(\frac{1}{\rho^{2}} \frac{\partial \Psi}{\partial x} - \frac{1}{\rho} \frac{\partial^{2} \Psi}{\partial \rho \partial x} \right)$$
(18)

Membrane analysis of the scalloped shell, therefore, reduces to the mathematical problem of solving equation (15) subject to appropriate boundary conditions. The equation, as can be seen, is a linear second-order partial differential equation with variable coefficients, and includes all the derivatives of the stress function Ψ up to the second order. It does not seem possible to reduce it to a known standard form, though it can be solved, as will be shown in appendix A, by the method of separation of variables.

In the next two sections, solutions of equation (15) will be discussed in detail, along with numerical results, for two particular types of structures, namely the single scallop and the parachute type structure. The latter is so called because of its similarity to a parachute in surface geometry, loading and boundary conditions. In each case, the numerical results for a similar shell of revolution will also be given for comparison purposes. It will be seen that the basic mathematical approach outlined in appendix A applies equally to both cases and yields solutions that are very similar in form.

THE SINGLE SCALLOP

Consider the symmetric scalloped shell shown in figure 3. It will be assumed for this particular example that the edge members in the vertical planes $\theta=\pm\theta_0$ cannot resist any thrust normal to their planes, so that $N_\theta=0$ along these edges. Furthermore, it will be assumed that $N_r=0$ along the edge r=a. It must be mentioned that with the technique to be presented in this section, it is also possible to prescribe other distributions of the force N_r along this boundary.

Since the structure, as well as the loading, is symmetric about the vertical plane θ = 0, it is sufficient to assume a solution symmetric in θ and confine the analysis to the domain $0 \le r \le a$, $0 \le \theta \le \theta_0$. In terms of the stress function Ψ and the variables ρ, x , the mathematical problem of solving equilibrium equation (15), subject to the boundary conditions $N_{\theta}(r,\theta_0)$ = 0, $N_r(a,\theta)$ = 0, can be described as follows:

$$L(\Psi) = F(\rho, x)$$
 $0 \le \rho \le 1$; $0 \le x \le x_0$ (19a)

$$\frac{\partial^2 \Psi}{\partial \rho^2} = \mu \rho^2 x^2 \quad \text{at} \quad x = x_0 \tag{19b}$$

$$\left[\frac{1}{\rho}\frac{\partial\Psi}{\partial\rho} + \frac{\beta}{\rho^2}\frac{\partial^2\Psi}{\partial x^2} - \mu\rho^2\left(\frac{2}{3} + x^2\right)\right] = 0 \quad \text{at} \quad \rho = 1$$
 (19c)

The variables in equation (19a) can be separated by assuming solutions of the form ρ^{λ_k} $\phi_k(x)$, where the functions $\phi_k(x)$ satisfy a linear ordinary second-order differential equation with variable coefficients and can be obtained in the form of an infinite series in even powers of x. By use of this approach,4 the solution $\Psi(\rho,x)$ can be obtained in the form:

$$\Psi(\rho, \mathbf{x}) = \Psi_{\mathbf{p}}(\rho, \mathbf{x}) + \Psi_{\mathbf{H}}(\rho, \mathbf{x}) \tag{20}$$

The function Ψ_D in (20) satisfies both the inhomogeneous differential equation (19a) and the inhomogeneous boundary condition (19b), and has the form

$$\Psi_{p}(\rho,x) = \frac{1}{2\mu} \rho^{2} \sum_{n=0,2,4,...} \alpha_{n}x^{n} + \mu\rho^{4} \sum_{n=0,2,4,...} \beta_{n}x^{n}$$
(21)

The homogeneous solution $\Psi_{H}(\rho,x)$ has the form

$$\Psi_{\mathrm{H}}(\rho,\mathbf{x}) = \sum_{\mathrm{m=1,2,3,...}} A_{\mathrm{m}} \Psi_{\mathrm{Hm}}(\rho,\mathbf{x})$$
 (22)

where each function Ψ_{Hm} , defined by

$$\Psi_{\rm Hm} = \rho^{\lambda_{\rm m}} F_{\rm m}(\mathbf{x}) \tag{23}$$

satisfies both the homogeneous differential equation

$$L(\Psi_{Hm}) = 0 (24)$$

and the homogeneous boundary condition

$$\frac{\partial^2}{\partial \rho^2} \left(\Psi_{\text{Hm}} \right) = 0 \tag{25}$$

The values λ_m are the eigenvalues of the system described by equations (24) and (25). These eigenvalues occur, in the present case, as pairs of positive and negative numbers of equal magnitude, so that the corresponding homogeneous solutions take on the form ρ^{λ_m} $f_m(x)$, $\rho^{-\lambda_m}$ $g_m(x)$, where

⁴For more details see appendix A.

$$f_{m}(x) = \sum_{n=0,2,4,...} c_{m,n}x^{n}$$

$$g_{m}(x) = \sum_{n=0,2,4,...} c_{m,n}x^{n}$$
(26)

The homogeneous solution can now be written as

$$\Psi_{H} = \sum_{m=1,2,3,...} A_{m} \rho^{\lambda_{m}} \sum_{n=0,2,4,...} C_{m,n} x^{n} + \sum_{m=1,2,3,...} B_{m} \rho^{-\lambda_{m}} \sum_{n=0,2,4,...} D_{m,n} x^{n}$$
(27)

Substituting equations (21) and (27) into equation (20), the solution Ψ may be written

$$\Psi(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{2} \sum_{\mathbf{n} = 0, 2, 4, \dots} \alpha_{\mathbf{n}} \mathbf{x}^{\mathbf{n}} + \mu \rho^{4} \sum_{\mathbf{n} = 0, 2, 4, \dots} \beta_{\mathbf{n}} \mathbf{x}^{\mathbf{n}}
+ \sum_{\mathbf{m} = 1, 2, 3, \dots} A_{\mathbf{m}} \rho^{\lambda_{\mathbf{m}}} \sum_{\mathbf{n} = 0, 2, 4, \dots} C_{\mathbf{m}, \mathbf{n}} \mathbf{x}^{\mathbf{n}}
+ \sum_{\mathbf{m} = 1, 2, 3, \dots} B_{\mathbf{m}} \rho^{-\lambda_{\mathbf{m}}} \sum_{\mathbf{n} = 0, 2, 4, \dots} D_{\mathbf{m}, \mathbf{n}} \mathbf{x}^{\mathbf{n}}$$
(28)

Applying (28) and definitions (18), the stress resultants can be expressed as follows:

$$n_{r} = \frac{1}{2\mu} \sum_{n=0,2,4,...} [2\alpha_{n} + \beta(n+2)(n+1)\alpha_{n+2}]x^{n}$$

$$+ \mu\rho^{2} \sum_{n=0,2,4,...} [4\beta_{n} + \beta(n+2)(n+1)\beta_{n+2}]x^{n} - \mu\rho^{2}(\frac{2}{3} + x^{2})$$

$$+ \sum_{m=1,2,3,...} A_{m}\rho^{\lambda_{m}-2} \sum_{n=0,2,4,...} [\lambda_{m}C_{m,n} + \beta(n+2)(n+1)C_{m,n+2}]x^{n}$$

$$+ \sum_{m=1,2,3,...} B_{m}\rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} [-\lambda_{m}D_{m,n} + \beta(n+2)(n+1)D_{m,n+2}]x^{n}$$

$$(29)$$

$$n_{\theta} = \frac{1}{2\mu} \sum_{n=0,2,4,...} 2\alpha_{n}x^{n} + \mu\rho^{2} \left(\sum_{n=0,2,4,...} 12\beta_{n}x^{n} - x^{2} \right)$$

$$+ \sum_{m=1,2,3,...} A_{m}\rho^{\lambda_{m}-2} \sum_{n=0,2,4,...} \lambda_{m}(\lambda_{m} - 1)C_{m,n}x^{n}$$

$$+ \sum_{m=1,2,3,...} B_{m}\rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} \lambda_{m}(\lambda_{m} + 1)D_{m,n}x^{n}$$

$$(30)$$

$$n_{r\theta} = \sqrt{\beta} \left[\frac{1}{2\mu} \sum_{n=0,2,4,...} - n\alpha_{n} x^{n-1} + \mu \rho^{2} \sum_{n=0,2,4,...} - 3n\beta_{n} x^{n-1} \right]$$

$$+ \sum_{m=1,2,3,...} A_{m} \rho^{\lambda_{m-2}} \sum_{n=0,2,4,...} n(1 - \lambda_{m}) C_{m,n} x^{n-1}$$

$$+ \sum_{m=1,2,3,...} B_{m} \rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} n(1 + \lambda_{m}) D_{m,n} x^{n-1}$$
(31)

It should be observed that the homogeneous solutions associated with the negative eigenvalues are singular at the point $\rho=0$, which is an included point of the domain. Consequently, the undetermined coefficients B_m associated with these solutions must be chosen as zero. The coefficients A_m can be chosen appropriately to prescribe suitable edge forces at $\rho=1$ (corresponding to r=a). In particular it is possible to choose these coefficients so as to satisfy the condition prescribed in (19c). For paraboloids of revolution $(\beta=0)^5$ the eigenfunctions $f_m(x)$ form a set of orthogonal trigonometric functions, and the coefficients A_m can be conveniently determined by means of this orthogonality property. In the present case $(\beta \neq 0)$, no such orthogonality property is readily available, and it is necessary to resort to suitable numerical methods such as collocation, matching coefficients of power series, or mean square approximation. The last technique has been found to give the most satisfactory results, and its application to the case of scalloped paraboloids is described briefly in appendix B.

⁵Transformation (11) for this case is $\theta = x$.

Consider, as a particular example, the case β = 1, μ = 0.5, θ_0 = $\pi/6$. The homogeneous solution (22) was found to be sufficiently approximated by the six-term truncated series

$$\Psi_{H} = \sum_{m=1,2,...}^{6} A_{m} \Psi_{Hm} = \sum_{m=1,2,...}^{6} A_{m} \rho^{\lambda_{m}} \sum_{n=0,2,4,...} C_{m,n} x^{n}$$
(32)

where the eigenvalues λ_m and the coefficients A_m are listed in table I. These values have been obtained according to the methods discussed in the appendixes A and B. The coefficients α_n , β_n associated with the particular solution (21) and the coefficients $C_{m,n}$ associated with the homogeneous solutions Ψ_{Hm} may then be evaluated by means of the recursion relationships given in appendix A. In this example, the first 25 terms of each of the series expansions in x in equations (29) - (31) were used to obtain the stress resultants presented in figures 4-7.

TABLE I.- SINGLE SCALLOP: EIGENVALUES $\left|\lambda_{m}\right|$ AND THE UNDETERMINED COEFFICIENTS A_{m}

m	$ \lambda_{m} $	A _m
1 2 3 4 5 6	2.26514 6.63706 11.0383 15.4446 19.8525 24.2612	0.453853 913284×10 ⁻² .689597×10 ⁻³ 121309×10 ⁻³ .134767×10 ⁻⁴ 286088×10 ⁻⁴

The broken line in figure 4 represents the meridional force $n_{\rm r}$ at the edge ρ = 1 due to the particular solution $\Psi_{\rm p};$ the solid line represents the force $n_{\rm r}$ due to this particular solution plus the homogeneous solution $\Psi_{\rm H}$ (eq. (32)). This homogeneous solution is added to obtain the desired vanishing edge force $n_{\rm r}$ along the edge ρ = 1. The difference between the broken and solid lines in figure 4 indicates the efficiency of this edge correction by the six-term approximation (32). This efficiency would improve as more solutions $\Psi_{\rm Hm}$ were included in (32). The forces $n_{\theta},\,n_{\rm r\theta}$ along this edge are plotted in figure 5.

For the interior points of the domain, it can be shown that only the first few homogeneous solutions are of importance, since the solutions corresponding to higher eigenvalues rapidly diminish as ρ decreases from $\rho=1$. The solid lines in figures 6 and 7 represent the stress resultants in the scallop, while the broken lines represent the stress resultants in the corresponding shell of revolution ($\beta=0$). The forces $n_{\bf r},\,n_{\theta}$ along the central meridian (fig. 6) are lower in most parts of the scalloped shell than in the shell of revolution. The same is true for shear stress along the edge $\theta=\theta_0$, although along this edge the meridianal force $n_{\bf r}$ is higher in the scalloped shell for a considerable distance from the apex.

It should be observed that for both types of shells the stresses tend to be relatively high in the vicinity of the three corners. These stress concentrations are pronounced, in the case of the scalloped shells, near the apex, and in the case of the shells of revolution, near the lower two corners. For either type of structure the corner regions need special attention and should be designed to withstand higher stresses than the rest of the shell.

The method of analysis for a shell terminated at an inner boundary r = b \neq 0 (ρ = ρ_{0} \neq 0) would be similar. In this event, however, the solutions corresponding to the negative eigenvalues would have to be included in the analysis, since these solutions are necessary for prescribing appropriate boundary values at r = b. If the lowest eigenvalue $\left|\lambda_{1}\right|$ is sufficiently high, then the coefficients $A_{m},$ B_{m} associated with the two sets of homogeneous solutions can be determined separately, by mean square approximation at each of the two boundaries. Otherwise, it may be necessary to use an iterative scheme, similar to the one described in reference 8, in order to obtain a satisfactory solution.

It should be noted at this point that the bulge parameter β has been prescribed unity only for the purpose of the numerical example. The technique places no restriction on the magnitude of this parameter, although, of course, it must always be positive, since surfaces with negative β belong to a different family.

PARACHUTE TYPE STRUCTURE

The method demonstrated in the preceding section can also be applied to the parachute type structure with an opening at the top, as shown in figure 8. In this case, however, the circumferential force n_θ need no longer be zero at $\theta=\pm\theta_0$. Because of structural symmetry and symmetric loading it is sufficient to solve the problem, as for the single scallop, in the domain $b\leq r\leq a,\,0\leq\theta\leq\theta_0$.

The boundary condition at $\theta = \theta_0$ can be obtained as follows. If the rope is considered an individual structure, loaded by the membrane forces from the shell at the edge $\theta = \theta_0$, it can be shown that the rope tension T, at any coordinate r, must satisfy the equilibrium condition (see fig. 9),

$$T = \frac{\left[1 + \frac{4A^{2}r^{2}(1 + \beta\theta_{0}^{2})^{2}}{2A(1 + \beta\theta_{0}^{2})}\right]^{3/2}}{2A(1 + \beta\theta_{0}^{2})}$$

$$= \frac{\frac{4Apa\beta\theta_{0}r[1 + \frac{4A^{2}r^{2}(1 + \beta\theta_{0}^{2})^{2}}{2A(1 + \beta\theta_{0}^{2})}]^{1/2}}{2A(1 + \beta\theta_{0}^{2})}$$
(33)

On the other hand, considering the entire structure above a given radius r, it follows that the rope tension T and the membrane forces n_r , $n_{r\theta}$ must also satisfy the condition of equilibrium of vertical forces:

$$T = \frac{p[1 + \frac{1}{4}A^{2}r^{2}(1 + \beta\theta_{0}^{2})^{2}]^{1/2}}{2A(1 + \beta\theta_{0}^{2})r} \left[\frac{\pi r^{2}}{N} - \frac{P}{pN} - \frac{P}{pN} \right]$$

$$- \frac{1}{4aAr^{2}} \int_{0}^{\theta_{0}} (1 + \beta\theta^{2})n_{r} d\theta - \frac{1}{4}Aar^{2} \int_{0}^{\theta_{0}} \beta\theta n_{r\theta} d\theta$$
(34)

where P is an arbitrary vertical load concentrated downward at the apex, and N is the number of scallops. The load P may be assigned any value appropriate to the problem of interest. Eliminating the rope tension T between equations (33) and (34), the boundary condition at $\theta = \theta_{\rm O}$ is obtained:

$$r^{2}\beta\theta_{0}n_{\theta}(r,\theta_{0}) + r^{2}\int_{0}^{\theta_{0}} [(1+\beta\theta^{2})n_{r} + \beta\theta n_{r\theta}]d\theta = \frac{\pi r^{2}}{4ANa} - \frac{P}{4ApaN}$$
(35)

The boundary conditions at the edges $\, r = b$ and $\, r = a \,$ depend on the nature of the structure. For a model parachute it would be reasonable, for example, to expect an almost vanishing meridional force $\, n_{\rm r} \,$ along these boundaries: such a surface would be expected to carry the applied loads by means of tensile and shear forces alone, since the material may be assumed incapable of sustaining compressive stresses. Assuming uniform normal pressure, the governing differential equation, condition (35) and the above requirements can be described as follows:

$$L(\Psi) = F(\rho, x) \qquad \rho_0 \le \rho \le 1 ; \qquad 0 \le x \le x_0$$
 (36)

$$\rho^{2}\beta \mathbf{x}_{O}\left(\frac{\partial^{2}\Psi}{\partial\rho^{2}}\right)_{\mathbf{x}=\mathbf{x}_{O}} + \rho^{2}\int_{0}^{\mathbf{x}_{O}}\left[\left(1 + \mathbf{x}^{2}\right)\left(\frac{1}{\rho}\frac{\partial\Psi}{\partial\rho} + \frac{\beta}{\rho^{2}}\frac{\partial^{2}\Psi}{\partial\mathbf{x}^{2}}\right) + \beta\mathbf{x}\left(\frac{1}{\rho^{2}}\frac{\partial\Psi}{\partial\mathbf{x}} - \frac{1}{\rho}\frac{\partial^{2}\Psi}{\partial\rho\partial\mathbf{x}}\right)\right]d\mathbf{x}$$

$$= \frac{\mathbf{x}_{O}\rho^{2}}{4\mu} + \mu\rho^{4}\left[\frac{2}{3}\mathbf{x}_{O} + \left(\beta + \frac{5}{9}\right)\mathbf{x}_{O}^{3} + \frac{\mathbf{x}_{O}^{5}}{5}\right] - \frac{P\sqrt{\beta}}{4\rho\mathbf{a}^{2}\mu\mathbf{N}}$$

$$(37)$$

$$n_r \approx 0$$
 at $\rho = \rho_0 (r = b)$ (38)

$$n_r \approx 0$$
 at $\rho = 1 (r = a)$ (39)

$$n_{\mathbf{r}}$$
, $n_{\theta} \ge 0$ (40)

The solution of equations (36), by the method described in appendix A, can be written in the form:

$$\Psi(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{2} \sum_{\mathbf{n}=0,2,4,...} \alpha_{\mathbf{n}} \mathbf{x}^{\mathbf{n}} - \frac{\mathbf{k}}{2\mu} \log(\rho \sqrt{1 + \mathbf{x}^{2}})
+ \mu \rho^{4} \sum_{\mathbf{n}=0,2,4,...} \beta_{\mathbf{n}} \mathbf{x}^{\mathbf{n}} + \sum_{\mathbf{m}=1,2,3,...} A_{\mathbf{m}} \rho^{\lambda_{\mathbf{m}}} \sum_{\mathbf{n}=0,2,4,...} C_{\mathbf{m},\mathbf{n}} \mathbf{x}^{\mathbf{n}}
+ \sum_{\mathbf{m}=1,2,3,...} B_{\mathbf{m}} \rho^{-\lambda_{\mathbf{m}}} \sum_{\mathbf{n}=0,2,4,...} D_{\mathbf{m},\mathbf{n}} \mathbf{x}^{\mathbf{n}}
= \Psi_{\mathbf{p}} + \Psi_{\mathbf{H}}$$
(41)

where

$$\Psi_{p} = \frac{\rho^{2}}{2\mu} \sum_{n=0,2,4,...} \alpha_{n} x^{n} + \mu \rho^{4} \sum_{n=0,2,4,...} \beta_{n} x^{n} - \frac{k}{2\mu} \log(\rho \sqrt{1 + x^{2}})$$

$$\Psi_{H} = \sum_{m=1,2,3,...} A_{m} \rho^{\lambda_{m}} \sum_{n=0,2,4,...} C_{m,n} x^{n} + \sum_{m=1,2,3,...} B_{m} \rho^{\lambda_{m}} \sum_{n=0,2,4,...} D_{m,n} x^{n}$$

$$k = \frac{P \sqrt{\beta}}{2pa^{2}N} \left[\frac{1}{(1 - x_{0})\beta + \beta \tan^{-1} x_{0} + (x_{0}^{3}/3)} \right] \qquad (42)$$

A comparison of equations (41) with (28) reveals that the forms of the two solutions differ only in the term $k/2\log(\rho\sqrt{1+x^2})$. By means of equations (41), the membrane forces n_r , n_θ , $n_{r\theta}$ and the rope tension T can be expressed as follows:

$$\begin{split} n_{r} &= -\frac{k}{2\mu} \frac{1}{\rho^{2}} \left[1 + \frac{\beta(1-x^{2})}{(1+x^{2})^{2}} \right] + \frac{1}{2\mu} \sum_{n=0,2,4,...} [2\alpha_{n} + \beta(n+2)(n+1)\alpha_{n+2}]x^{n} \\ &+ \mu\rho^{2} \left\{ \sum_{n=0,2,4,...} [4\beta_{n} + \beta(n+2)(n+1)\beta_{n+2}]x^{n} - \left(\frac{2}{3} + x^{2}\right) \right\} \\ &+ \sum_{m=1,2,3,...} A_{m}\rho^{\lambda_{m}-2} \sum_{n=0,2,4,...} [\lambda_{m}C_{m,n} + \beta(n+2)(n+1)C_{m,n+2}]x^{n} \\ &+ \sum_{m=1,2,3,...} B_{m}\rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} [-\lambda_{m}D_{m,n} + \beta(n+2)(n+1)D_{m,n+2}]x^{n} \end{split}$$

(43)

$$n_{\theta} = \frac{k}{2\mu} \frac{1}{\rho^{2}} + \frac{1}{2\mu} \sum_{n=0,2,4,...} 2\alpha_{n}x^{n} + \mu\rho^{2} \left(\sum_{n=0,2,4,...} 12\beta_{n}x^{n} - x^{2} \right)$$

$$+ \sum_{m=1,2,3,...} A_{m}\rho^{\lambda_{m}-2} \sum_{n=0,2,4,...} \lambda_{m}(\lambda_{m} - 1)C_{m,n}x^{n}$$

$$+ \sum_{m=1,2,3,...} B_{m}\rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} \lambda_{m}(\lambda_{m} + 1)D_{m,n}x^{n}$$

$$(44)$$

$$n_{r\theta} = \sqrt{\beta} \left[-\frac{k}{2\mu} \frac{1}{\rho^{2}} \frac{x}{1 + x^{2}} - \frac{1}{2\mu} \sum_{n=0,2,4,...} n\alpha_{n}x^{n-1} - \mu\rho^{2} \sum_{n=0,2,4,...} 3n\beta_{n}x^{n-1} \right]$$

$$+ \sum_{m=1,2,3,...} A_{m}\rho^{\lambda_{m}-2} \sum_{n=0,2,4,...} n(1 - \lambda_{m})C_{m,n}x^{n}$$

$$+ \sum_{m=1,2,3,...} B_{m}\rho^{-(\lambda_{m}+2)} \sum_{n=0,2,4,...} n(1 + \lambda_{m})D_{m,n}x^{n}$$

$$(45)$$

$$T = 2pa^{2}\sqrt{\beta} x_{0} \rho \frac{\left[1 + 4\mu^{2}\rho^{2}(1 + x_{0}^{2})^{2}\right]^{1/2}}{(1 + x_{0}^{2})} n_{\theta}(\rho, x_{0})$$
 (46)

The boundary conditions (38) and (39) and the requirement (40) represent perhaps an ideal situation, but the tensile requirement (40) could be satisfied even if the forces n_r were tensile at the upper and lower edges.

The stresses due to the particular solution Ψ_p are of interest in view of requirement (40). In the present investigation it has been found that for a given choice of the parameters N, $\rho_{\rm O}(<\!\!<\!\!1)$ and μ the above requirement will be satisfactorily met if β and k are less than certain readily determined critical values $\beta_{\rm Cr}$ and $k_{\rm Cr}.$ The significance of these parameters will be discussed later. It should be observed in passing that for practical parachute models N is generally large (say >10). In the present study it has been found that values of $|\lambda_m|$ depend largely on N and are of the order of magnitude mN. For large N, the particular solution Ψ_p then becomes the dominant solution, and the approximate solution $\Psi_{\rm E}$ can be used to obtain satisfactory values for stresses in most parts of the shell. Consider, as a numerical example, a parachute model with 12 scallops (N = 12), $\rho_{\rm O}$ = 0.1 and μ = 0.4. For this combination of the geometric parameters, the critical values of β , k have been determined to be 0.5 and 0.0017, respectively, and the corresponding solution $\Psi_{\rm D}$ meets requirements (36) - (38). It may be

recalled here that, as for the single scallop, the two sets of homogeneous solutions associated with the positive and negative eigenvalues are to be used to satisfy proper edge conditions at the lower and upper boundaries. Since condition (38) at the upper edge is already satisfactorily met, the coefficients B_m , associated with the solutions in $\rho^{-\lambda_m}$, may be appropriately chosen as zero. Furthermore, with $\beta=\beta_{\rm Cr}$, the edge forces $n_{\rm r}$ at $\rho=1$ due to the particular solution are already very small, and a five-term truncated series for Ψ_H satisfactorily meets requirement (39). The corresponding five values of λ_m , A_m are listed in table II.

TABLE II.- PARACHUTE TYPE STRUCTURE: THE CONSTANT k , THE EIGENVALUES $\left|\lambda_m\right|$ AND THE UNDETERMINED COEFFICIENTS A_m

k	_	\cap	$\cap \cap$	ם ר	726
ĸ.	_	v.	\cup	1	120

m	$ \lambda_{\rm m} $	A _m
1 2 3 4 5	12.7379 23.3730 33.5535 43.5804 53.5374	-0.248942×10 ⁻³ .200055×10 ⁻⁴ 488396×10 ⁻⁵ .179266×10 ⁻⁵ 11563 ⁴ ×10 ⁻⁵

Before discussing the results for this case and a similar shell of revolution, it is necessary to contrast certain features of the solutions for the two types of structures. For both the scalloped shell and the shell of revolution it is possible to find a solution $\Psi_{\rm D}$ that meets requirements (36)-(38). In the shell of revolution, the functions $f_{\rm m}$ form a set of orthogonal trigonometric functions. The coefficients $\ensuremath{A_{\text{M}}}$ can then be readily determined, from this orthogonality property, to prescribe appropriate edge forces at ρ = 1. Along this edge the force n_r due to the particular solution Ψ_p is equal to $(1-\rho_0^2)/4\mu$, and the dimensionless sum of its vertical components along this edge is a downward force of magnitude $\theta_0(1-\rho_0^2)$. The sum of the vertical components of the force n_r due to the homogeneous solution Ψ_H vanishes along this edge. This implies that a proper boundary condition at $\rho = 1$ consists of a distribution of the force n_r , which has the same vertical resultant as that due to the particular solution. Two such distributions are shown in figures 10(a) and 10(b). For the scalloped model, on the other hand, it is not necessary to follow the same pattern because the corresponding homogeneous solutions do contribute a vertical resultant. In this case, however, the coefficients Am have to be determined by numerical techniques, with resulting numerical difficulties if N is large and if a large number of terms of the homogeneous solution are needed to prescribe vanishing edge force n_r at $\rho = 1$. However, if β is close to β_{cr} , then only a few terms of the homogeneous solution would be adequate to meet requirement (39). This is true because in the scalloped model, as will be seen later, the forces nr decrease as β increases and become very small at the base when β is close

to β_{cr} . In the shell of revolution, on the other hand, these forces increase toward the base and become almost constant in the lower parts of the shell.

The membrane forces n_r , n_θ , $n_{r\theta}$ and the rope tension T presented in figures 11-14 have been obtained by means of the first 25 terms of each of the series in x in expressions (43) - (46). The broken lines in these figures represent the membrane forces in a similar shell of revolution subjected to the boundary condition in figure 10(b) for $\xi=0.1$. It can be seen from these figures that, for equilibrium of the shell, all three stress resultants n_r , n_θ , $n_{r\theta}$ are necessary. For the particular parameters chosen the force n_r decreases toward the edge $\rho=1$. This drop in n_r is more noticeable along the central meridian than along the edge $\theta=\theta_0$ (fig. 12). The hoop force n_θ at a given radius (fig. 13) remains nearly constant although its magnitude varies with the radius and reaches a peak value at the edge $\rho=1$. The shear forces are relatively low, except in the lower half of the shell near the edge $\theta=\theta_0$. The rope tension T (fig. 14) has a maximum value at the base ($\rho=1$) and decreases steadily toward the apex.

It is interesting to note how the membrane forces in the scalloped shell differ from those in a similar shell of revolution (figs. 11 and 12). It can be seen that the forces n_r in the scalloped shell are significantly lower than those in the shell of revolution. The forces n_θ are also lower but not as significantly as the forces n_r . The shear force $n_{r\theta}$ in the shell of revolution vanishes at $\theta=\theta_0$, whereas in the scalloped shell a relatively low but nonvanishing shear force is necessary at this edge. The behavior of the forces n_r , n_θ in the shell of revolution along $\theta=\theta_0$ is also worth noting: These forces have high magnitudes in the neighborhood of the lower corners, and n_θ even changes sign and becomes compressive.

It should be mentioned, at this point, that the solution $k/2\mu \log (\rho \sqrt{1+x^2})$, with k=0.0017 in the particular example, corresponds to a downward load of magnitude $0.35\pi b^2 p$ at the apex. In figure 8, the ropes, but not the scallops, are continued to the apex. Prescribing a value for k therefore amounts to assuming a certain amount of tension in these ropes at $\rho=\rho_0$. In the particular example considered, if k>0.0017 the edge forces n_r at $\rho=\rho_0$ due to Ψ_p would be compressive, while if k<0.0017 these forces would be tensile. In either case, appropriate values for n_r would have to be prescribed along this edge by means of the homogeneous solutions associated with the negative eigenvalues.

It was mentioned earlier in this section that in order to meet the tensile requirement (40), the parameter β should be less than $\beta_{cr}.$ This can be illustrated in figure 15 where membrane forces due to the dominant solution Ψ_p are plotted along the central meridian for different values of $\beta.$ The constant k in each case has been chosen equal to k_{cr} to meet requirement (38). This figure shows that when β is very small the membrane forces approach those in a corresponding shell of revolution (β = 0). However, as β increases the stress resultant n_r decreases markedly, and, for β = 0.6, even changes sign and becomes compressive in the region $\rho \geq 0.75.$ The hoop force n_θ decreases also, although not as significantly as the meridional force $n_r.$ It can be seen that when β is less than 0.5, the

shell (and a corresponding parachute) could support the applied uniform pressure by means of tensile and shear forces alone.

SCOPE AND LIMITATIONS

The stress function approach is valid as long as there exists a one-to-one correspondence between points of the middle surface and their projections on a reference plane. The scalloped paraboloids (eq. (9)) meet this requirement. The mathematical approach in appendix A for finding this stress function Y relies on two important conditions. First, the differential equation and the boundary conditions must be such that the method of separation of variables can be applied. Second, the solutions involving the variable x must be obtainable in the form of an infinite series in powers of x. These requirements are fulfilled in the problems considered so far. It is of interest to mention other problems where these requirements may be met.

Consider, for example, the more general class of surfaces

$$Z = Ar^{n}(1 + \beta\theta^{2}) \tag{47}$$

of which the scalloped paraboloids constitute a special case. These surfaces meet the above requirements for application of the stress function approach. When such shells are subjected to uniform normal pressure, the stress function Ψ must satisfy the differential equation.

$$\left(\frac{n}{2} + \beta + \frac{n}{2} x^2\right) \frac{\partial^2 \Psi}{\partial \rho^2} - 2(n-1)\beta x \frac{1}{\rho} \frac{\partial^2 \Psi}{\partial \rho \partial x} + \frac{1}{2} n(n-1)(1+x^2) \frac{\beta}{\rho^2} \frac{\partial^2 \Psi}{\partial x^2}$$

$$+\frac{1}{2}$$
 n(n - 1)(1 + x^2) $\frac{1}{\rho}$ $\frac{\partial \Psi}{\partial \rho}$ + 2(n - 1) βx $\frac{1}{\rho^2}$ $\frac{\partial \Psi}{\partial x}$

$$= \frac{1}{2\mu} \rho^{2-n} + \mu \rho^{n} \left[\frac{n^{3}}{n+1} + \left(n^{2} \frac{2n+1}{n+1} + 3\beta \right) x^{2} + n^{2} x^{4} \right]$$
 (48)

where the membrane forces are related to the stress function Ψ as follows:

$$n_{\mathbf{r}\theta} = \frac{1}{\rho} \frac{\partial \Psi}{\partial \rho} + \frac{\beta}{\rho^{2}} \frac{\partial^{2} \Psi}{\partial \mathbf{x}^{2}} - \mu \rho^{n} \left(\frac{n}{n+1} + \mathbf{x}^{2} \right)$$

$$n_{\theta} = \frac{\partial^{2} \Psi}{\partial \rho^{2}} - \mu \rho^{n} \mathbf{x}^{2}$$

$$n_{r\theta} = \sqrt{\beta} \left(\frac{1}{\rho^{2}} \frac{\partial \Psi}{\partial \mathbf{x}} - \frac{1}{\rho} \frac{\partial^{2} \Psi}{\partial \rho \partial \mathbf{x}} \right)$$

$$(49)$$

It can be verified that the boundary conditions, similar to those for the two cases considered, are such that the method of appendix A can be applied to equation (48) to obtain solution Ψ in the form:

$$\Psi(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{\alpha} F_{\alpha}(\mathbf{x}) + \mu \rho^{\gamma} F_{\beta}(\mathbf{x}) + \frac{\mathbf{k}}{2\mu} \rho^{\delta} F_{\gamma}(\mathbf{x}) + \Psi_{H}(\rho, \mathbf{x})$$
 (50)

where

$$\alpha = 4 - n$$

$$\gamma = n + 2$$

$$\delta = 2 - n$$
(51)
$$\begin{cases}
= 0 \text{ for a single scallop} \\
\neq 0 \text{ for a parachute model}
\end{cases}$$

When $\delta=0$ (or n=2) the third term of (50) is replaced by $k/2\mu \log (\rho \sqrt{1+x^2})$.

It should be pointed out that it is not generally possible to find a solution in the form of equation (50) if any of the indices α , γ , δ is equal to an eigenvalue of the corresponding eigenvalue problem. For example, λ = 1 happens to be an eigenvalue for all n; hence, difficulties may be encountered when n = 1 and n = 3. In order to predict whether a solution in the form of equation (50) does or does not exist for a given problem, it is necessary to investigate each individual case further. It is interesting to note that since the shells corresponding to n > 2 become relatively flat in the polar regions, the membrane forces must be high in these regions in order to balance the applied normal pressure. For such cases the structure would require additional reinforcement in the upper regions.

CONCLUDING REMARKS

In the preceding sections it has been shown that the stress function approach can be successfully used in solving problems of scalloped paraboloids. It has been further demonstrated that the surface parameters associated with these problems may profoundly affect the application of the scalloped shell – particularly where a compressionless state is desired. The two scalloped structures considered in detail in this report are characterized by the parameters N, μ , ρ_0 , β , and defined by equation (9). It has been shown for the parachute model that for a given large N and given values of the parameters ρ_0 , μ , it is possible to use k = k_{Cr} in the particular solution Ψ_p to determine the maximum allowable bulge parameter $\beta_{\rm Cr}$, beyond which the meridional stress could become compressive. It was found in the particular example considered that for given N (= 12) and ρ_0 (= 0.1), the greater the value of μ , the smaller the value of $\beta_{\rm Cr}$. It would be useful to carry this

investigation further and plot relationships between $~\mu$ and $\beta_{\mbox{cr}}~$ for different sets of $~\rho_{\mbox{O}}$ and N. Such plots could serve as a useful aid in the design of parachute-type structures.

As mentioned earlier in this report, the only difference between the forms of the solutions for the two cases considered is the term $k/2\mu$ log $(\rho\sqrt{1+x^2})$, which is singular at the apex. For the single scallop, this term is absent. For the parachute model shown in figure 8 this term is used to prescribe a certain amount of tension in the ropes in the region $\rho<\rho_0$. Solutions of this type may be useful in other aspects of parachute analysis, as well.

In the case of the single scallop the character of the stresses does not seem to be radically affected by the magnitude of the bulge parameter β ; but such is not the case with the parachute type structure, as can be seen in figure 15. The latter structure, when designed to the critical β , has a lower hoop stress and a much lower meridional stress than the corresponding shell of revolution. In comparison with shells of revolution, indications are that the scalloped shell may be an efficient structural form warranting further study. It would be useful indeed to extend the analysis to include bending, deformation, and material properties, and to consider the scalloped shell for other structural applications such as pressure vessel bulkheads.

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National Aeronautics and Space Administration
Moffett Field, Calif., 94035, May 7, 1968
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APPENDIX A

THE SOLUTION Y FOR THE SINGLE SCALLOP

PARTICULAR SOLUTION

Consider the differential equation for the single scallop,

$$L[\Psi(\rho, \mathbf{x})] = F(\rho, \mathbf{x}) ; \quad 0 \le \rho \le 1 , \quad 0 \le \mathbf{x} \le \mathbf{x}_0$$
 (A1)

subject to the boundary condition

$$\left(\frac{\partial^2 \Psi}{\partial \rho^2} - \mu \rho^2 x^2\right) = 0 \quad \text{at} \quad x = x_0 \tag{A2}$$

The operator L and the function F are defined in equations (16) and (17). Let

$$\Psi(\rho, \mathbf{x}) = \Psi_{\mathbf{p}}(\rho, \mathbf{x}) + \Psi_{\mathbf{H}}(\rho, \mathbf{x}) \tag{A3}$$

where Ψ_p satisfies equation (Al) and the boundary condition (A2). If Ψ_p is assumed in the separated form,

$$\Psi_{p}(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{2} F_{\alpha}(\mathbf{x}) + \mu \rho^{4} F_{\beta}(\mathbf{x})$$
 (A4)

then $\ F_{\alpha}\text{, }F_{\beta}$ must satisfy the equations

$$\beta(1 + x^{2})F_{\alpha}^{"} - 2\beta xF_{\alpha}^{'} + 2(2 + \beta + 2x^{2})F_{\alpha} = 1$$
(A5)

$$\beta(1 + x^2)F_{\beta}^{"} - 6\beta xF_{\beta}^{'} + 4(4 + 3\beta + 4x^2)F_{\beta} = \frac{8}{3} + \left(3\beta + \frac{20}{3}\right)x^2 + 4x^4$$

$$0 \le x \le x_0 \tag{A6}$$

and the boundary conditions

$$F_{\alpha}(x_{O}) = 0 \tag{A7}$$

$$F_{\beta}(x_0) = \frac{x_0^2}{12} \tag{A8}$$

When used as function superscripts, the primes denote appropriate differentiation with respect to \mathbf{x} .

Solutions for $F_{\alpha}(x)$, $F_{\beta}(x)$ can be found in the form of a convergent series containing terms in even powers of x as follows:

$$F_{\alpha}(x) = f_{\alpha}(x) + \mu_{\alpha}g_{\alpha}(x) \tag{A9}$$

$$F_{\beta}(x) = f_{\beta}(x) + \mu_{\beta}g_{\beta}(x) \tag{Alo}$$

where μ_{α} , μ_{β} are constants to be determined and

$$f_{\alpha}(x) = \sum_{\substack{0,2,4,\dots}} \alpha_n^{\prime} x^n \tag{All}$$

$$f_{\beta}(\mathbf{x}) = \sum_{\substack{0,2,4,\dots}} \beta_{n}^{\dagger} \mathbf{x}^{n}$$
 (Al2)

$$g_{\alpha}(\mathbf{x}) = \sum_{\substack{0,2,4,\dots}} \alpha_{n}^{"} \mathbf{x}^{n}$$
 (Al3)

$$g_{\beta}(x) = \sum_{0,2,4,\dots} \beta_{n}^{n} x^{n}$$
 (Al4)

The functions f_{α} , f_{β} must satisfy the inhomogeneous equations (A5) and (A6), while g_{α} , g_{β} must satisfy the corresponding homogeneous equations. The coefficients α_n , α_n , β_n , β_n can then be obtained from the resulting recurrence relationships,

$$\alpha_{n+2}^{i} = \frac{1}{\beta(n+2)(n+1)} \left\{ \gamma_{n} - [n(n-3)\beta + 2(2+\beta)]\alpha_{n}^{i} - 4\alpha_{n-2}^{i} \right\}$$
 (A15)

$$\beta_{n+2}'' = \frac{1}{\beta(n+2)(n+1)} \left\{ \gamma_n' - [n(n-7)\beta + 4(4+3\beta)]\beta_n' - 16\beta_{n-2}' \right\}$$
 (A16)

$$\alpha_{n+2}^{"} = -\frac{1}{\beta(n+2)(n+1)} \left\{ [n(n-3)\beta + 2(2+\beta)]\alpha_{n}^{"} + 4\alpha_{n-2}^{"} \right\}$$
 (A17)

$$\beta_{n+2}^{"} = -\frac{1}{\beta(n+2)(n+1)} \left\{ [n(n-7)\beta + 4(4+3\beta)]\beta_{n}^{"} + 16\beta_{n-2}^{"} \right\}$$
 (A18)

for
$$n = 0, 2, 4, ...$$

where

and

The constants of integration μ_{α} , μ_{β} in (A9) and (A10) are obtained by requiring that F_{α} , F_{β} satisfy conditions (A7) and (A8), so that

$$\mu_{\alpha} = \frac{-\sum_{\substack{O,2,4,\dots\\O,2,4,\dots}} \alpha_{n}^{n} x_{O}^{n}}{\sum_{\substack{O,2,4,\dots\\O}} \alpha_{n}^{n} x_{O}^{n}}$$
(A20)

$$\mu_{\beta} = \frac{\left(x_{O}^{2}/12\right) - \Sigma \beta_{n}^{t} x_{O}^{n}}{\Sigma \beta_{n}^{t} x_{O}^{n}} \tag{A21}$$

Defining the coefficients $\ \alpha_n^{},\ \beta_n^{}$ by

$$\alpha_{n} = \alpha_{n}^{\prime} + \mu_{\alpha} \alpha_{n}^{\prime\prime} \tag{A22}$$

$$\beta_n = \beta_n'' + \mu_B \beta_n'' \qquad n = 0, 2, 4, \dots$$
 (A23)

The solutions $\ F_{\alpha}$ and F_{β} can be finally written in the form:

$$F_{\alpha}(x) = \sum_{n=0,2,4,\dots} \alpha_n x^n$$
 (A24)

$$F_{\beta}(\mathbf{x}) = \sum_{n=0,2,4,\dots} \alpha_n \mathbf{x}^n \tag{A25}$$

HOMOGENEOUS SOLUTION

Consider a homogeneous solution

$$\Psi_{Hm}(\rho,x) = \rho^{\lambda_m} f_m(x)$$

which satisfies

$$L(\Psi_{Hm}) = 0 (A26)$$

and the boundary conditions

$$\frac{\partial^2}{\partial \rho^2} \Psi_{\text{Hm}}(\rho, \mathbf{x}) = 0 \quad \text{at} \quad \mathbf{x} = \mathbf{x}_0$$
 (A27)

The function $f_m(x)$, then, must satisfy the equation

$$\beta(1 + x^{2})f_{m}^{n} - 2\beta(\lambda_{m} - 1)xf_{m}^{n} + \lambda_{m}[\beta(\lambda_{m} - 1) + \lambda_{m}(1 + x^{2})]f_{m} = 0$$
 (A28)

subject to the condition

$$\lambda_{m}(\lambda_{m} - 1)f_{m}(x_{O}) = 0 \quad \text{at} \quad x = x_{O}$$
 (A29)

A nontrivial solution for $f_m(x)$ exists only when λ_m equals an eigenvalue of the problem defined by equations (A28) and (A29). The values λ_m = 0 and λ_m = 1 are obviously two eigenvalues. However, the corresponding homogeneous solutions make no contribution to the membrane forces n_r , n_θ , $n_{r\theta}$. The eigenvalues of interest, therefore, are associated with the condition

$$f_{m}(x_{O}) = 0 (A30)$$

It should be observed that the solution for f_m , when one exists, can be obtained through the method of infinite series, that is, $f_m(x)$ may be expanded in the form,

$$f_{m}(x) = \sum_{n=0,2,4,...} c_{m,n} x^{n}$$
 (A31)

where

$$C_{m,0} = 1$$
 $C_{m,n} = 0$ for $n < 0$

and

$$C_{m,n+2} = \frac{-1}{(n+1)(n+2)} \left\{ \left[n(n+1) - 2n(\lambda_m-1) + \left(1 + \frac{1}{\beta} \right) \lambda_m^2 - \lambda_m \right] C_{m,n} + \frac{\lambda_m^2}{\beta} C_{m,n-2} \right\}$$
for $n = 0, 2, 4, \dots$ (A32)

Equation (A31) can be applied to (A30) to obtain an infinite series in powers of λ_m . The zeroes of this infinite series are the required eigenvalues. From a practical standpoint, the leading eigenvalues can be found by truncating such a series after a finite number of terms and then finding the roots of the resulting polynomial in λ_m . In the present work, it was found that

for large $N(=\pi/\theta_O)$ the polynomial approach gives convergent values only for the first one or two zeroes of the infinite series. A more satisfactory approach in this case has been found to be the method of interpolation or the false position method (ref. 9). It must be mentioned, however, that this method is to be applied to equation (A30) in a special way and not to the truncated polynomial. The process essentially consists of selecting a set of values for λ_m at chosen fixed intervals, obtaining the coefficients $C_{m,n}$ from (A32), and using these $C_{m,n}$ in (A31) to evaluate each $f_m(x_O)$. The approximate zeroes of (A29) are then easily located and it is only necessary to repeat the entire process a suitable number of times in order to converge to suitably accurate roots. In order to use this approach most effectively, it is advantageous to have prior information about the character of the eigenvalues λ_m . In this connection the following discussion will be of interest.

If, instead of using the form (A31), $f_m(x)$ is expressed in the following form:

$$f_{\rm m}(x) = (1 + x^2)^{\frac{\lambda_{\rm m} - 1}{2}} u_{\rm m}(x)$$
 (A33)

then it can be verified that u_m must satisfy

$$u_{m}^{"} + \left[\frac{\lambda_{m}^{2} - 1}{(1 + x^{2})^{2}} + \frac{\lambda_{m}^{2}}{\beta} \right] u_{m} = 0$$
 (A34)

$$u_{m}(x_{O}) = 0 \tag{A35}$$

Furthermore, if λ_m is a complex eigenvalue and u_m the corresponding complex eigenfunction, then their complex conjugates $\overline{\lambda}_m$, \overline{u}_m also satisfy equations (A34) and (A35).

Using u_m , \overline{u}_m in Green's first formula (ref. 10)

$$\int_{0}^{x_{O}} u_{m}^{"}\overline{u}_{m} dx = \left(u_{m}^{"}\overline{u}_{m}\right)_{0}^{x_{O}} - \int_{0}^{x_{O}} u_{m}^{"}\overline{u}_{m}^{"} dx$$
(A36)

and realizing that $u_m(x_0) = 0$ and (because of symmetry) $u_m'(0) = 0$, equation (A36) reduces to

$$\int_{O}^{\mathbf{x}_{O}} \mathbf{u}_{\mathbf{m}}^{\mathbf{n}} \overline{\mathbf{u}}_{\mathbf{m}} d\mathbf{x} = - \int_{O}^{\mathbf{x}_{O}} \mathbf{u}_{\mathbf{m}}^{\mathbf{n}} \overline{\mathbf{u}}_{\mathbf{m}}^{\mathbf{n}} d\mathbf{x}$$
 (A37)

Substituting for $u_m^{"}$ from equation (A34) and rearranging yields

$$\lambda_{m}^{2} = \frac{\int_{0}^{x_{0}} u_{m}^{\dagger} \overline{u}_{m}^{\dagger} dx + \int_{0}^{x_{0}} \frac{1}{(1 + x^{2})^{2}} u_{m} \overline{u}_{m} dx}{\int_{0}^{x_{0}} \left[\frac{1}{(1 + x^{2})^{2}} + \frac{1}{\beta} \right] u_{m} \overline{u}_{m} dx}$$
(A38)

where $\beta > 0$ by definition.

It is evident from the above expression that, since u_m is a nontrivial solution, λ_m^2 must always be positive definite. Hence, the eigenvalues are always real, occurring in pairs of positive and negative values of equal magnitudes. Consequently, the homogeneous solution Ψ_H can be written as

$$\Psi_{\mathrm{H}}(\rho,\mathbf{x}) = \sum_{\mathrm{m=1,2,3,...}} \left[A_{\mathrm{m}} \rho^{\lambda_{\mathrm{m}}} f_{\mathrm{m}}(\mathbf{x}) + B_{\mathrm{m}}^{-\lambda_{\mathrm{m}}} g_{\mathrm{m}}(\mathbf{x}) \right]$$
(A39)

where the functions f_m , g_m correspond to the eigenvalues $+\lambda_m$ and $-\lambda_m$, respectively. The function f_m is obtainable in the form defined in equation (A31), and g_m is obtainable in the form

$$g_{m}(x) = \sum_{n=0,2,4,...} D_{m,n}x^{n}$$
 (A40)

where the coefficients $D_{m,n}$ can be generated from the recurrence relation (A32) by means of the negative value $-\lambda_m$. The stress function can then be written as

$$\Psi(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{2} \sum_{\mathbf{n} = 0, 2, 4, \dots} \alpha_{\mathbf{n}} \mathbf{x}^{\mathbf{n}} + \mu \rho^{4} \sum_{\mathbf{n} = 0, 2, 4, \dots} \beta_{\mathbf{n}} \mathbf{x}^{\mathbf{n}}
+ \sum_{\mathbf{m} = 1, 2, 3, \dots} A_{\mathbf{m}} \rho^{\lambda_{\mathbf{m}}} \sum_{\mathbf{n} = 0, 2, 4, \dots} C_{\mathbf{m}, \mathbf{n}} \mathbf{x}^{\mathbf{n}}
+ \sum_{\mathbf{m} = 1, 2, 3, \dots} B_{\mathbf{m}} \rho^{-\lambda_{\mathbf{m}}} \sum_{\mathbf{n} = 0, 2, 4, \dots} D_{\mathbf{m}, \mathbf{n}} \mathbf{x}^{\mathbf{n}}$$
(A41)

The homogeneous solutions associated with the negative eigenvalues tend to infinity as ρ approaches zero. Hence, the coefficients B_m associated with these solutions must be chosen to be zero if ρ = 0 is to be an included point of the domain. If the domain is terminated at an inner boundary ρ = ρ_O (corresponding to r = b), then the coefficients B_m may be different

from zero, and their values would have to be determined by resorting to an appropriate numerical technique, such as approximation in the mean, together with an iterative scheme.

THE SOLUTION Y FOR THE PARACHUTE TYPE STRUCTURE

For the parachute type structure the stress function Ψ must satisfy equation (36) subject to the condition (37). The solution for this case is similar to that for the single scallop and can be written as

$$\Psi(\rho, \mathbf{x}) = \frac{1}{2\mu} \rho^{2} \sum_{n=0,2,4,...} \alpha_{n} \mathbf{x}^{n} + \mu \rho^{4} \sum_{n=0,2,4,...} \beta_{n} \mathbf{x}^{n} - \frac{k}{2\mu} \log(\rho \sqrt{1 + \mathbf{x}^{2}})$$

$$+ \sum_{m=1,2,3,...} \left[A_{m} \rho^{\lambda_{m}} f_{m}(\mathbf{x}) + B_{m} \rho^{-\lambda_{m}} g_{m}(\mathbf{x}) \right]$$
(A42)

The coefficients α_n , β_n and the functions f_m , g_m can be found using the approach outlined earlier in this section, where μ_α and μ_β are expressed as follows:

$$\mu_{\alpha} = \frac{-\frac{1}{2} - \sum_{n=0,2,4,\dots} \left(3\beta\alpha_{n}^{"} - \frac{2+3\beta}{n+1}\alpha_{n}^{"} - \frac{2}{n+1}\alpha_{n-2}^{"}\right)x_{0}^{n}}{\sum_{n=0,2,4,\dots} \left(3\beta\alpha_{n}^{"} - \frac{2+3\beta}{n+1}\alpha_{n}^{"} - \frac{2}{n+1}\alpha_{n-2}^{"}\right)x_{0}^{n}}$$
(A43)

$$\mu_{\beta} = -\frac{\frac{2}{3} - \frac{5}{9} x_{0}^{2} - \frac{1}{5} x_{0}^{4} - \sum_{n=0,2,4,\dots} \left(5\beta\beta_{n}^{'} - \frac{\frac{1}{4} + 5\beta}{n+1} \beta_{n}^{'} - \frac{\frac{1}{4}\beta_{n-2}^{'}}{n+1} \right) x_{0}^{n}}{\sum_{n=0,2,4,\dots} \left(5\beta\alpha_{n}^{"} - \frac{\frac{1}{4} + 5\beta}{n+1} \beta_{n}^{"} - \frac{\frac{1}{4}\beta_{n-2}^{"}}{n+1} \right) x_{0}^{n}}$$
(A44)

The value of the constant k in (A42) depends on the tension in the ropes in the region $\rho \leq \rho_0$.

For the parachute type structure it can be verified that the function $\textbf{u}_{m}(\textbf{x})$ must satisfy the boundary condition

$$(\lambda_{\rm m}^2 - 1) x_{\rm O} u_{\rm m}(x_{\rm O}) + (1 + x_{\rm O}^2) u_{\rm m}^{\dagger}(x_{\rm O}) = 0$$
 (A45)

where

$$\lambda_{m}^{2} = \frac{\int_{0}^{x_{O}} u_{m}^{'} \overline{u}_{m}^{'} dx + \int_{0}^{x_{O}} \frac{u_{m} \overline{u}_{m} dx}{(1 + x^{2})^{2}} - \frac{x_{O}}{1 + x_{O}^{2}} u_{m}(x_{O}) \overline{u}_{m}(x_{O}) dx}{1 + x_{O}^{2}}$$

$$\int_{0}^{x_{O}} \left[\frac{1}{\beta} + \frac{1}{(1 + x^{2})^{2}} \right] u_{m} \overline{u}_{m} dx - \frac{x_{O}}{1 + x_{O}^{2}} u_{m}(x_{O}) \overline{u}_{m}(x_{O})$$
(A46)

It follows from equation (A46) that λ_m^2 must always be real. The positive definite character of λ_m^2 is difficult to establish in this case, but since the solutions corresponding to negative λ_m^2 have a rapidly oscillating singularity as ρ becomes small, the undetermined coefficients associated with these solutions may be chosen appropriately as zero. The eigenvalues of interest, then, are related to the positive values of λ_m^2 and are easily obtained by the interpolation procedure described earlier in this appendix.

APPENDIX B

MEAN SQUARE APPROXIMATION FOR COEFFICIENTS Am

The method of mean square approximation, used in determining the coefficients A_{m} of equations (28) and (41), will be described briefly in this appendix. Consider the case of the single scallop subject to the condition

$$\left[\frac{1}{\rho}\frac{\partial\Psi}{\partial\rho} + \frac{\beta}{\rho^2}\frac{\partial^2\Psi}{\partial x^2} - \mu\rho^2\left(\frac{2}{3} + x^2\right)\right] = 0$$
at $\rho = 1$
(B1)

Using Y, as defined in (28), the above condition can be expressed as

$$\sum_{m=1,2,3,...} A_{m} \sum_{n=0,2,4,...} [\lambda_{m} C_{m,n} + \beta(n+2)(n+1) C_{m,n+2}] x^{n}$$

$$+\frac{1}{2\mu}\sum_{n=0,2,4,\dots} [2\alpha_n + \beta(n+2)(n+1)\alpha_{n+2}]x^n$$

$$+ \mu \sum_{n=0,2,4,\dots} [4\beta_n + \beta(n+2)(n+1)\beta_{n+2}]x^n - \mu \left(\frac{2}{3} + x^2\right) = 0$$
 (B2)

or, more concisely, as

$$\sum_{m=1,2,3,...} A_{m}g_{m}(x) - y(x) = 0$$
(B3)

where

$$g_{m}(x) = \sum_{n=0,2,4,...} a_{m,n}x^{n}$$

$$y(x) = \sum_{n=0,2,4,...} \mu_{n}x^{n}$$
(B4)

The coefficients $a_{m,n}$, μ_n in equation (B)4) are defined by

$$a_{m,n} = \lambda_{m} C_{m,n} + \beta(n+2)(n+1)C_{m,n+2}$$

$$\mu_{n} = -\frac{1}{2\mu} \left[2\alpha_{n} + \beta(n+2)(n+1)\alpha_{n+2} \right]$$

$$-\mu[4\beta_{n} + \beta(n+2)(n+1)\beta_{n+2} - \delta_{n}]$$
(B5)

where

$$\delta_0 = \frac{2}{3}, \quad \delta_2 = 1$$

$$\delta_n = 0 \quad \text{for } n > 2$$
(B6)

The summation in (B3) is an infinite series, which includes correspondingly an infinite number of the undetermined coefficients A_m . For practical purposes this series is truncated after a suitable number of terms, k, and the coefficients A_m are determined to minimize the square of the difference between the sum Σ $A_m g_m(x)$ and the function y(x) in the interval 1,2,...k

 $0 \le x \le x_0$. This is the basic principle underlying the technique of mean square approximation (ref. 11). Its application to scalloped paraboloids can be described as follows:

Define

$$R(\mathbf{x}) = \sum_{m=1,2,3,\dots,k} A_m g_m(\mathbf{x}) - y(\mathbf{x})$$
(B7)

and

$$Q = \int_{0}^{\mathbf{x}_{O}} [\mathbf{R}(\mathbf{x})]^{2} d\mathbf{x} = \int_{0}^{\mathbf{x}_{O}} \left[\sum_{m=1,\dots,k} \mathbf{A}_{m} \mathbf{g}_{m}(\mathbf{x}) - \mathbf{y}(\mathbf{x}) \right]^{2} d\mathbf{x}$$
(B8)

For Q to be minimum the following conditions must be satisfied:

$$\frac{\partial}{\partial A_r} (Q) = 0 \quad \text{for } r = 1, 2, \dots, k$$
 (B9)

Hence, the condition

$$\int_{0}^{x_{0}} \left[\sum A_{m} g_{m}(x) g_{r}(x) \right] dx = \int_{0}^{x_{0}} y(x) g_{r}(x) dx \qquad r = 1, 2, \dots k$$
 (Blo)

or, interchanging the order of integration and summation,

$$\sum_{m=1,2,...} A_m \int_0^{x_0} g_m(x)g_r(x)dx = \int_0^{x_0} y(x)g_r(x)dx \qquad r = 1, 2, ... k$$
(B11)

Conditions (Bll) can be expressed as a set of $\,k\,$ simultaneous equations in unknown $\,A_m$, that is,

$$c_{r,m}A_m = B_r$$
 $r,m = 1, 2, ...k$ (B12)

where

$$c_{r,m} = \int_{0}^{x_{0}} g_{r}(x)g_{m}(x)dx = \sum_{s=0,2,4,...} \sum_{n=0,2,4,...} \frac{a_{r,s}a_{m,n}x_{0}}{s+n+1}$$
(B13)

$$B_{r} = \int_{0}^{x_{0}} y(x)g_{r}(x)dx = \sum_{s=0,2,4,...} \sum_{n=0,2,4,...} \frac{\mu_{n}a_{r,s}}{n+s+1} x_{0}^{s+n+1}$$
(B14)

for r, m = 1, 2, ... k

The unknown coefficients $\,{\rm A}_m\,$ are determined by solving the simultaneous equations (Bl2).

The results presented for the two examples have been obtained by the technique just described. When the structure has a boundary at $\rho=\rho_0$ (corresponding to r=b) and the magnitude of the lowest eigenvalue, $\left|\lambda_1\right|$, is sufficiently high, the homogeneous solutions are such that any disturbance produced at the edge $\rho=1$ decays before reaching the edge $\rho=\rho_0$. The method given here, then, can be used separately for correcting the value at each of the two boundaries, $\rho=\rho_0$ and $\rho=1$. In other cases it may be necessary to combine the present approach with an iterative scheme similar to the one described in reference 8.

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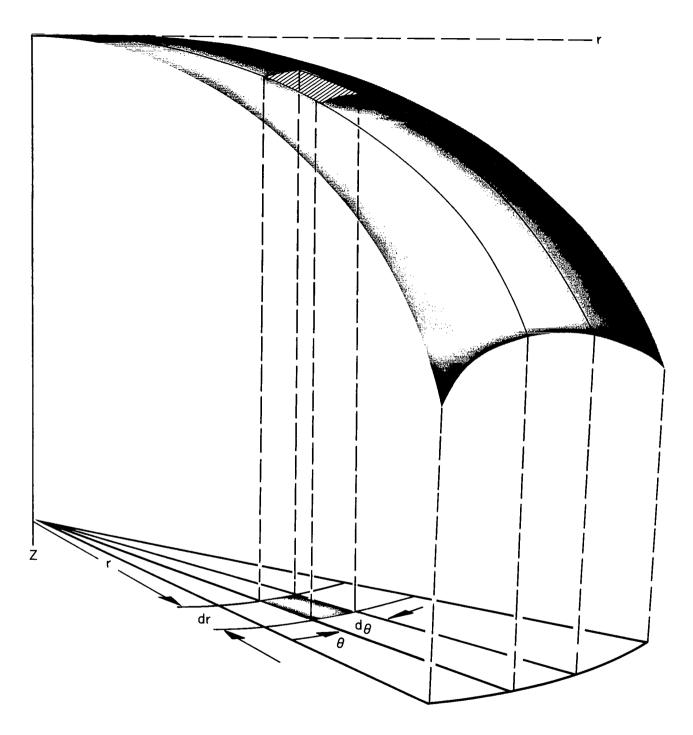


Figure 1.- Middle surface of the shell in cylindrical coordinates.

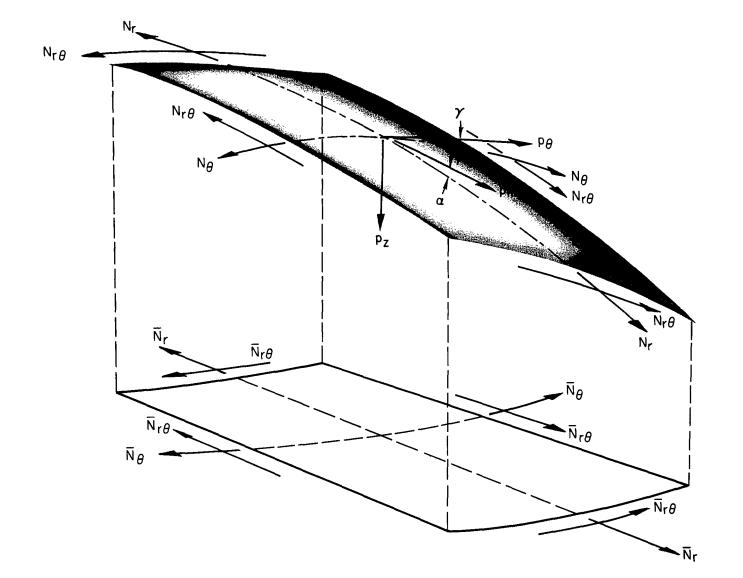


Figure 2.- Shell element.

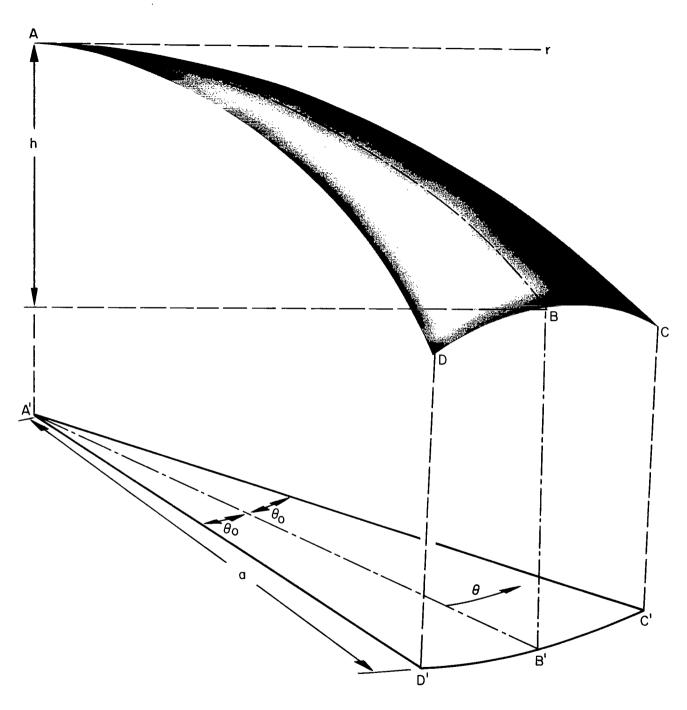


Figure 3.- Middle surface of the single scallop.

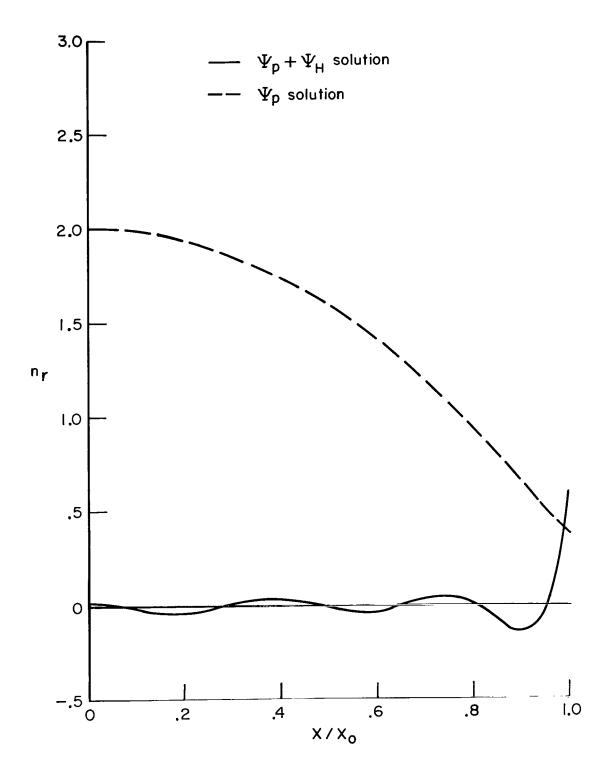


Figure 4.- The single scallop; membrane force $~n_{r}~$ along the edge; ρ = 1; $0 \le x \le x_{O}$

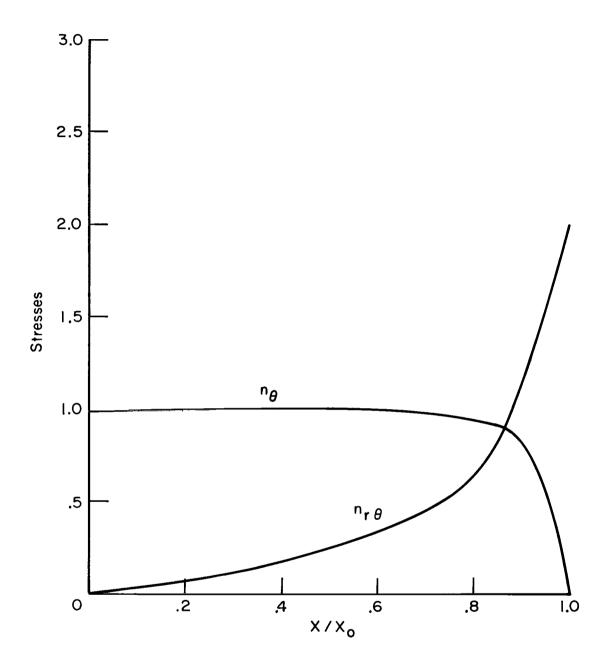


Figure 5.- The single scallop; membrane forces $~n_{\theta},~n_{r\theta}~$ along the edge; ρ = 1; $~0 \le x \le x_{O}.$

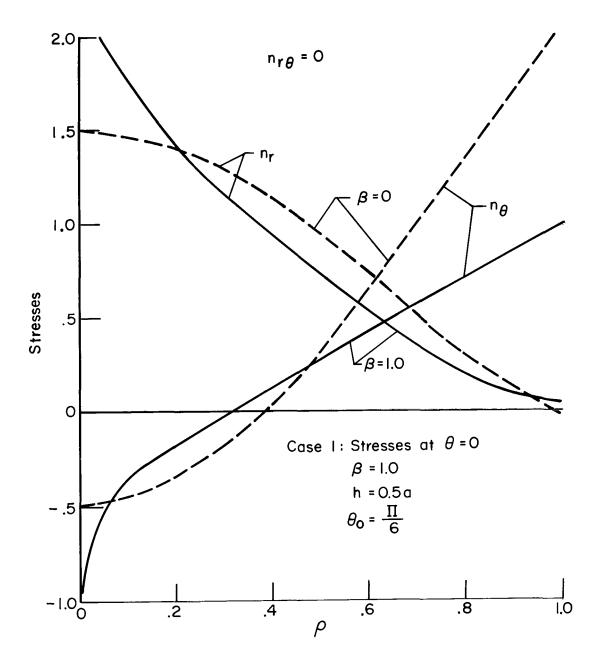


Figure 6.- The single scallop; membrane forces n_r , n_θ along the central meridian; x = 0; $0 \le \rho \le 1$; $n_{r\theta}$ = 0.

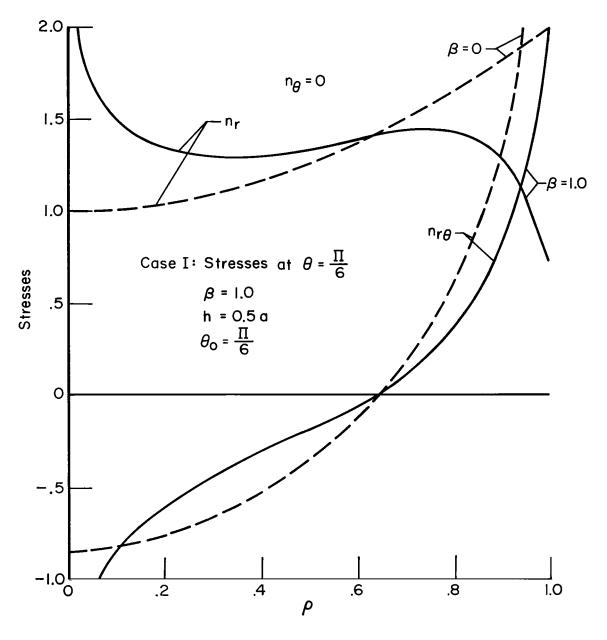


Figure 7.- The single scallop; membrane forces n_r , $n_{r\theta}$ along the edge; $x = x_0$; $0 \le \rho \le 1$; $n_{\theta} = 0$.

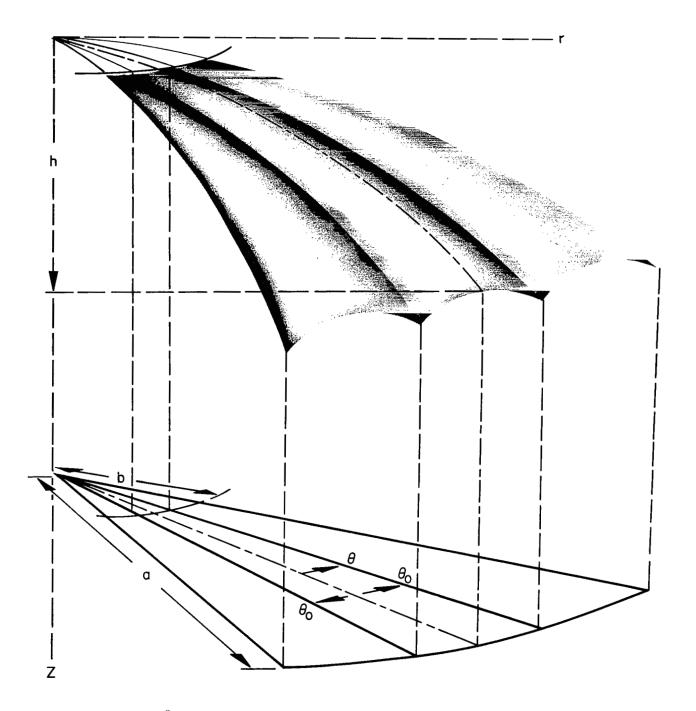


Figure 8.- Middle surface of the parachute type structure.

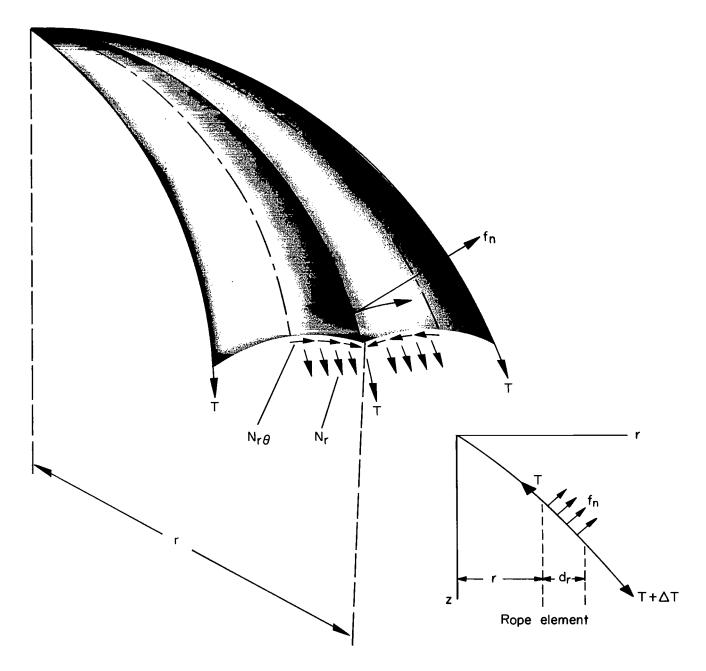
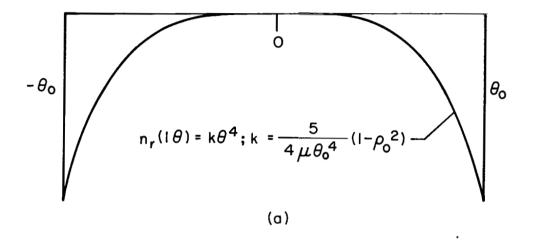


Figure 9.- Parachute type structure; condition for rope tension T at a radius r.

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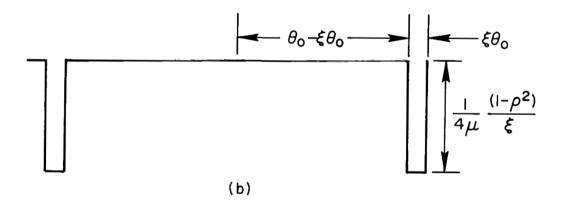


Figure 10.- Prescribable values of n_{r} (1, θ) in a shell of revolution.

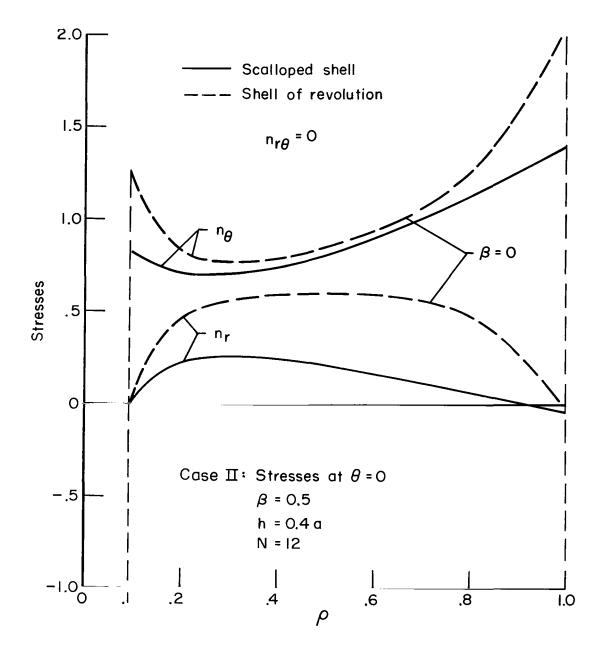


Figure 11.- Parachute type structure; membrane forces $n_{\bf r}$, n_{θ} along the central meridian; ρ_0 = 0.1 \leq ρ \leq 1; $n_{{\bf r}\theta}$ = 0.

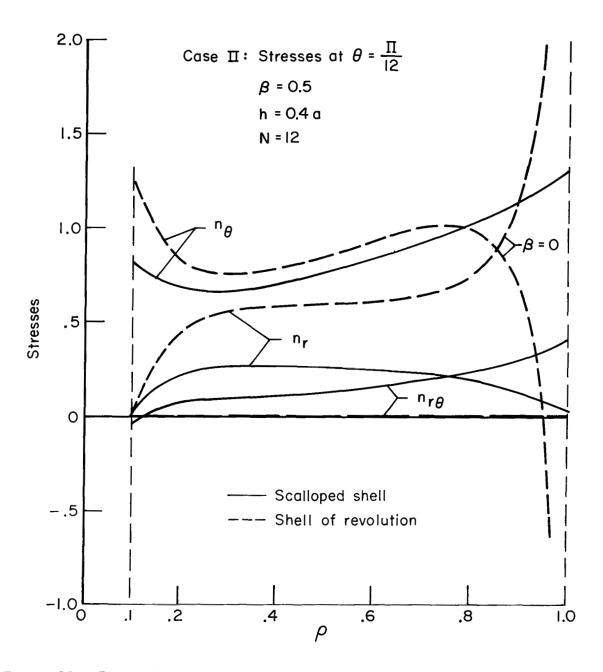


Figure 12.- Parachute type structure; membrane forces n_r , n_θ , $n_{r\theta}$ along the edge; $x=x_0$; $\rho_0=0.1 \le \rho \le 1$.

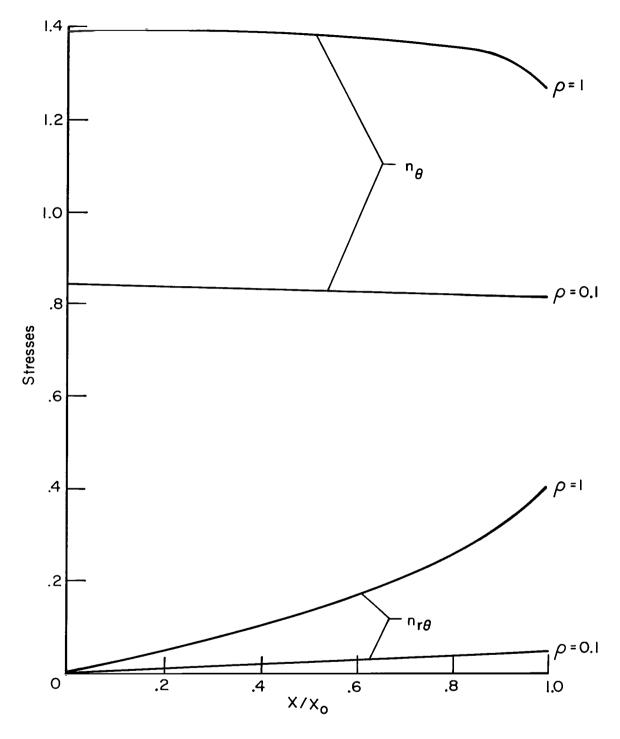


Figure 13.- Parachute type structure; membrane force $~n_{\theta}~$ along the edge; $~\rho$ = ρ_{O} = 0.1 and ρ = 1; 0 < x < x_{O} .

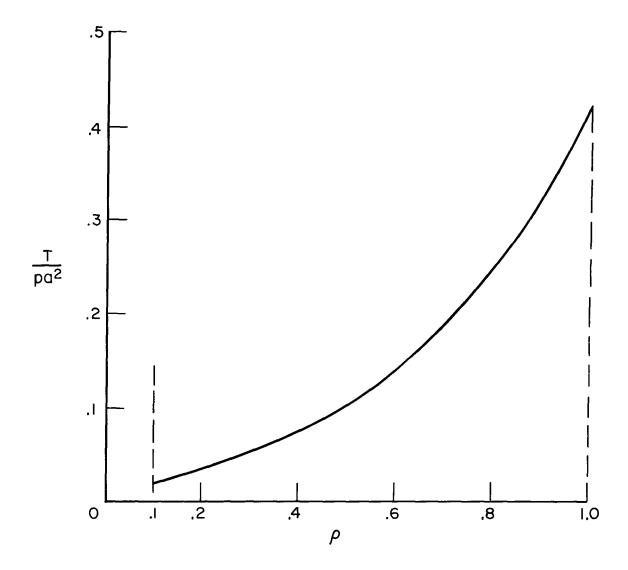


Figure 14.- Parachute type structure; rope tension T; $\rho_0 \le \rho \le 1$.

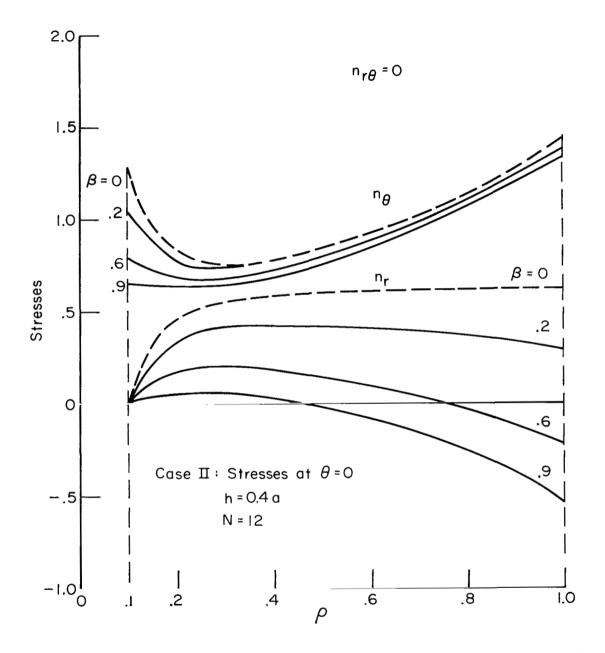


Figure 15.- Parachute type structure; membrane forces n_{r} , n_{θ} along the central meridian due to the solution Ψ_{p} for selected values of β ; $\rho_{o} \leq \rho \leq 1$; $n_{r\theta}$ = 0.

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